INSTYTUT TECHNIKI BUDOWLANEJ

Designated according to Article 29 of Regulation (EU) No 305/2011 and member of EOTA (European Organisation for Technical Assessment)

Member of EOTA

www.eota.eu

European Technical Assessment

ETA-14/0120 of 29/07/2024

General Part

Technical Assessment Body issuing the European Technical Assessment	Instytut Techniki Budowlanej
Trade name of the construction product	Injection system VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T for rebar connections
Product family to which the construction product belongs	Post-installed rebar connections with injection mortar
Manufacturer	ALSAFIX S.A.S. 114a rue Principale 67240 Gries, France
Manufacturing plant	ALSAFIX S.A.S. Manufacturing plant 1
This European Technical Assessment contains	23 pages including 3 Annexes which form an integral part of this Assessment
This European Technical Assessment is issued in accordance with regulation (EU) No 305/2011, on the basis of	European Assessment Document (EAD) 330087-01-0601 “Systems for post-installed rebar connection with mortar”
This version replaces	ETA-14/0120 issued on 07/10/2019

This European Technical Assessment is issued by the Technical Assessment Body in its official language. Translations of this European Technical Assessment in other languages shall fully correspond to the original issued document and shall be identified as such.

Communication of this European Technical Assessment, including transmission by electronic means, shall be in full. However, partial reproduction may only be made with the written consent of the issuing Technical Assessment Body. Any partial reproduction has to be identified as such.

Specific Part

1 Technical description of the product

The subject of this assessment are the post-installed connections, by anchoring or overlap connection joint of steel reinforcing bars (rebar) in existing structures made of normal weight concrete, using injection system VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T in accordance with the regulations for reinforced concrete construction.

Reinforcing bars made of steel with diameter from 8 to 32 mm and VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T injection mortars are used for the post-installed rebar connections. The steel element is placed into a drilled hole previously filled with an injection mortar and is anchored by the bond between embedded element, injection mortar and concrete.

An illustration and the description of the products are given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document (EAD)

The performances given in clause 3 are only valid if the post-installed rebar connections are used in the compliance with the specifications and conditions given in Annex B.

The provisions given in this European Technical Assessment are based on an assumed working life of the rebar connections of 50 and/or 100 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Performance of the product

3.1.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under static and quasi static loading	See Annex C1
Characteristic resistance under seismic loading	See Annex C2

3.1.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	See Annex C3

3.2 Methods used for the assessment

The assessment has been made in accordance with EAD 330087-01-0601.

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

According to Decision 96/582/EC of the European Commission the system 1 of assessment and verification of constancy of performance applies (see Annex V to regulation (EU) No 305/2011).

5 Technical details necessary for the implementation of the AVCP system, as provided in the applicable European Assessment Document (EAD)

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited in Instytut Techniki Budowlanej.

For type testing the results of the tests performed as part of the assessment for the European Technical Assessment shall be used unless there are changes in the production line or plant. In such cases the necessary type testing has to be agreed between Instytut Techniki Budowlanej and the notified body.

Issued in Warsaw on 29/07/2024 by Instytut Techniki Budowlanej

Anna Panek, MSc

Deputy Director of ITB

Examples of post-installed rebar connections

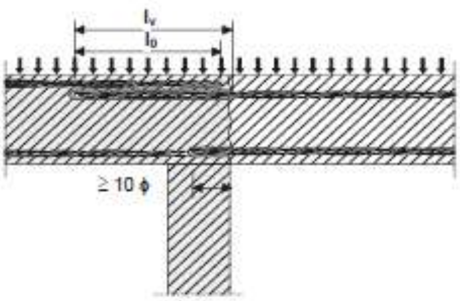


Figure 1.1 Overlap joint for rebar connections of slabs and beams

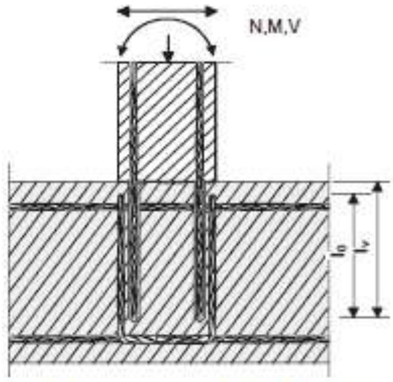


Figure 1.2 Overlap joint at a foundation of a column or wall where the rebar is stressed in tension

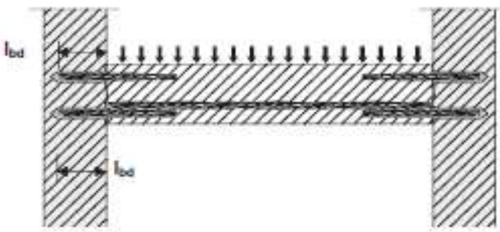


Figure 1.3 End anchoring of slabs or beams, designed as simply supported

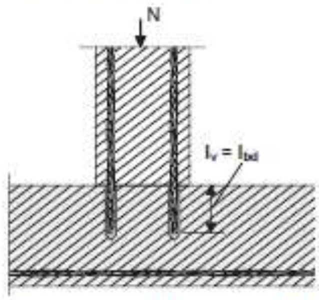
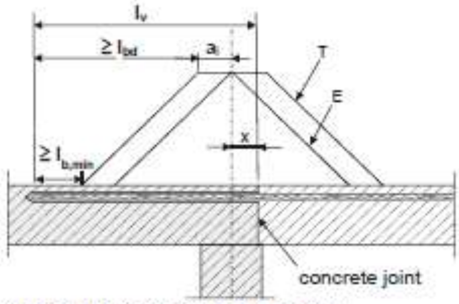


Figure 1.4 rebar connection for components stressed primarily in compression; rebar is stressed in compression



(only post-installed rebar is plotted)

Key to Figure 1.5

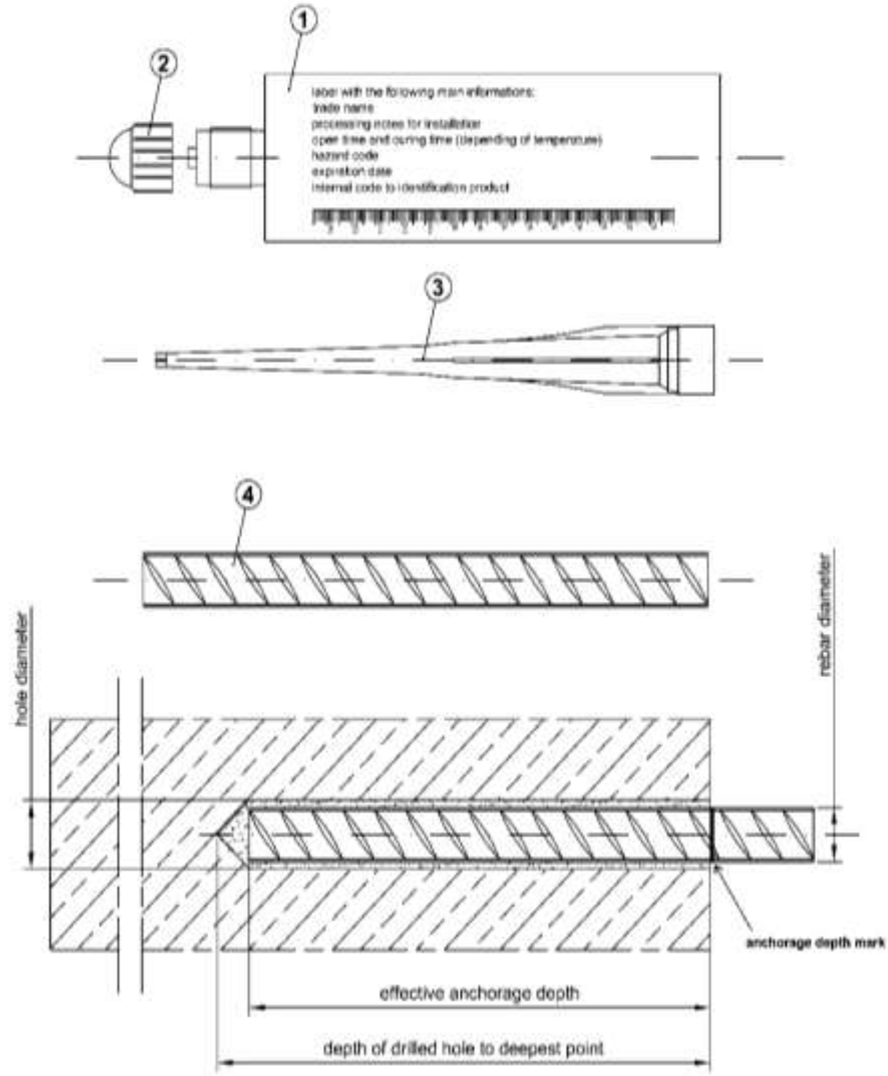
- T acting tensile force
- E envelope of $M_{Ed}/z + N_{Ed}$ (see EN 1992-1-1, Figure 9.2)
- x distance between the theoretical point of support and concrete joint

Note to Figure 1.1 to 1.5:

In the Figures no transverse reinforcement is plotted, the transverse reinforcement as required by EN 1992-1-1 shall be present.

The shear transfer between old and new concrete shall be designed according to EN 1992-1-1.

Figure 1.5 Anchoring of reinforcement to cover the line of acting tensile force



① **Cartridge**
② **Sealing cap**
③ **Mixer**
④ **Rebar - Reinforced bar**

Injection system VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T for rebar connections	**Annex A2** of European Technical Assessment ETA-14/0120
Product description Injection system	

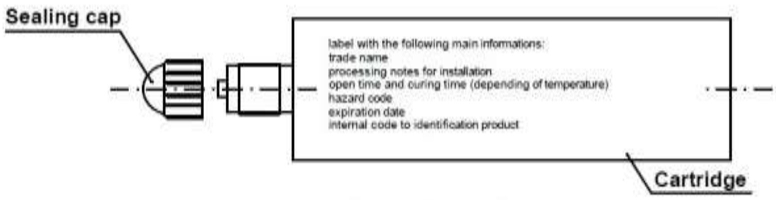
Table A1: Reinforcing bars (Rebar)

Designation	Material
Rebar according to EN 1992-1-1:2004+AC:2010	Bars and de-coiled rods Class B or C With f_{yk} and k according to EN 1992-1-1:2004+AC:2010 $f_{uk} = f_{tk} = k \cdot f_{yk}$ The rib height h: h ≤ 0,07 Ø

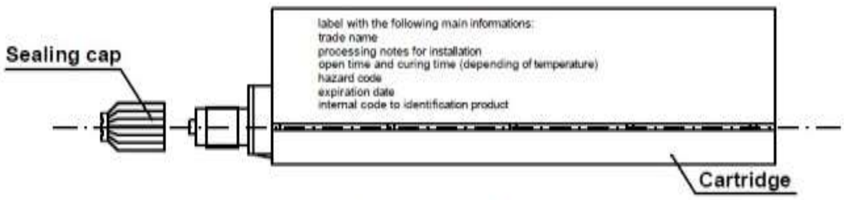
Table A2: Injection mortars

Product	Composition
VI 100-PRO VI 100-PRO-W VI 100-PRO-T (two component injection mortars)	Additive: quartz Bonding agent: vinyl ester resin styrene free Hardener: dibenzoyl peroxide

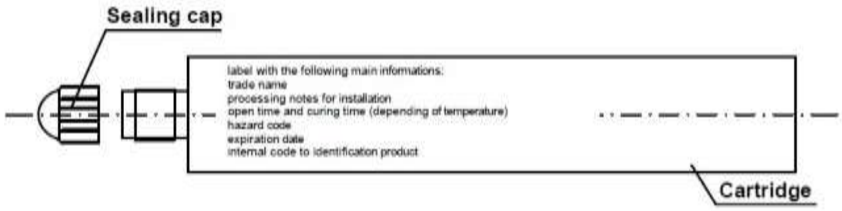
coaxial cartridge - sizes from 380 ml to 420 ml



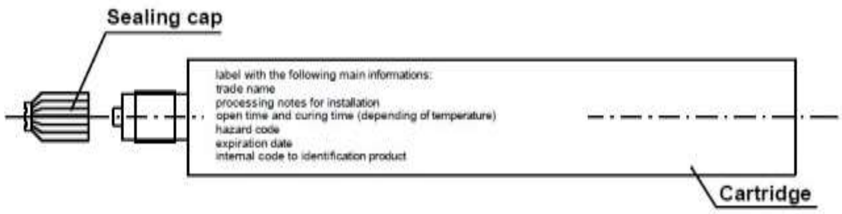
side by side cartridge - sizes from 345 ml to 825 ml



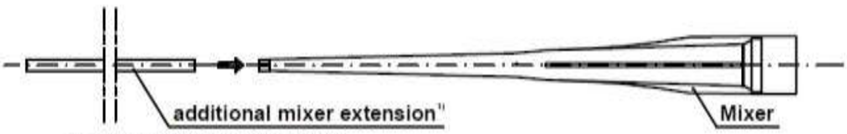
CIC foil cartridge - sizes from 165 ml to 300 ml



coaxial peeler cartridge - size of 280 ml



MIXER - the mixer is suitable for each type of cartridge



1) Variable length from 380 mm up to 1000 mm

Injection system VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T for rebar connections	**Annex A4** of European Technical Assessment ETA-14/0120
Product description Catridge types	

Specification of intended use

Anchorages subject to:

- Static and quasi-static load: from Ø8 to Ø32 mm.
- Seismic load: from Ø12 to Ø32 mm.
- Fire exposure: from Ø8 to Ø32 mm.

Working life:

- Working life: 50 and/or 100 years.

Base material:

- Reinforced or unreinforced normal weight concrete of strength class C12/15 at minimum to C50/60 at maximum according to EN 206 for static and quasi-static load and for fire exposure.
- Reinforced or unreinforced normal weight concrete of strength class C16/20 at minimum to C50/60 at maximum according to EN 206 for seismic load.
- Maximum chloride content of 0,40% (Cl 0,40) related to the cement content according to EN 206.
- Non-carbonated concrete.

  Note: In case of a carbonated surface of the existing concrete structure the carbonate layer shall be removed in the area of the post-installed rebar connection with a diameter of d_s + 60 mm prior to the installation of the new rebar. The depth of concrete to be removed shall correspond to at least the minimum concrete cover according to EN 1992-1-1:2004+AC:2010.

  The above may be neglected if building components are new and not carbonated and if building components are in dry conditions.

Temperature range:

The products may be used in the following temperature range:

- -40°C to +80°C (max. short term temperature +80°C and max. long term temperature +50°C).

Temperature of the base material according to Annex B4.

Design:

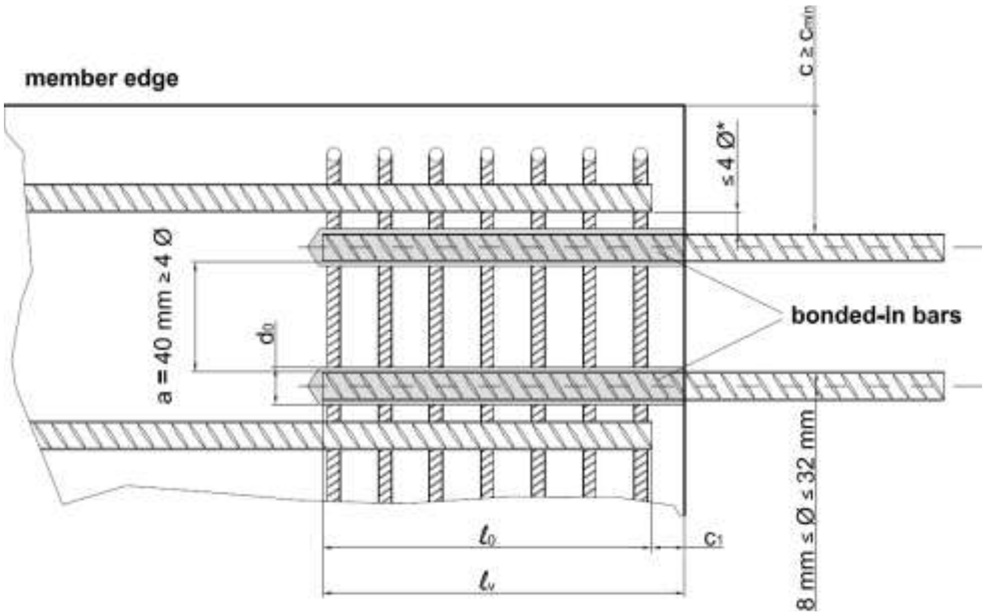
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking into account of the forces to be transmitted.
- Design according to EN 1992-1-1:2004+AC:2010 for static and quasi-static condition (see also Annex B2).
- Design according to EN 1998-1:2004+AC:2009 for seismic condition (see also Annex B2).
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing.

Installation:

- Dry or wet concrete.
- It must not be installed in flooded holes.
- Overhead installation is permissible.
- Hole drilling by hammer drill (HD), hollow drill bit (HDB) or compressed air drill (CA).
- Installation of the post-installed rebar shall be done only by suitable trained installer and under supervision on the site.
- Check the position of the existing rebar (if the position of existing rebar in not known it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint).

General design rules of construction for post-installed rebar

Post installed rebar may be designed for tension forces only.
The transfer of shear forces between new concrete and existing structure shall be designed additionally according to EN 1992-1-1:2004+AC:2010.
The joints for concreting must be roughened to at least such an extended that aggregate protrude.



* If the clear distance between overlapping rebar is greater than 4·Ø the overlap length shall be enlarged by the difference between the clear distance and 4·Ø.

l_0	lap length according to EN 1992-1-1:2004+AC:2010 for static and quasi-static loading or EN 1998-1:2004+AC:2009 for seismic loading
l_v	effective embedment depth; $l_v \geq l_0 + c_1$
c	concrete cover of post-installed rebar
c_{min}	minimum concrete cover according to Annex B3 and EN 1992-1-1:2004+AC:2010
c_1	concrete cover at end-face of existing rebar
d_0	nominal drill bit diameter according to Annex B3
Ø	rebar diameter (d_s)

Table B1-1: Installation parameters for static and quasi static loading

Rebar diameter [mm]	Ø8		Ø10		Ø12		Ø14	Ø16	Ø20	Ø22	Ø25	Ø28	Ø30	Ø32
Drill bit diameter [mm]	10[1)]	12[1)]	12[1)]	14[1)]	14[1)]	16[1)]	18	20	25	26	30	35	35	40
Brush diameter [mm]	12	14	14	16	16	18	20	22	27	27	32	37	37	42
Maximum embedment depth $l_{v,\,max}$ [mm]	250	400	250	500	250	600	700	800	1000	1000	1000	1000	1000	1000

[1)] Each of two given values can be used

Table B1-2: Installation parameters for seismic loading

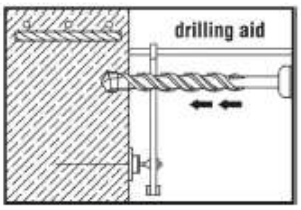
Rebar diameter [mm]	Ø12	Ø14	Ø16	Ø20	Ø22	Ø25	Ø28	Ø30	Ø32
Drill bit diameter [mm]	16	18	20	25	26	30	35	35	40
Brush diameter [mm]	18	20	22	27	27	32	37	37	42
Maximum embedment depth $l_{v,\,max}$ [mm]	600	700	800	1000	1000	1000	1000	1000	1000

Table B2: Minimum concrete cover c_{min} without drilling aid

Drilling method	Rebar diameter Ø	c_{min}
Hammer drilling (HD) Hollow drill bit (HDB)	< 25 mm	30 mm + 0,06 x l_v ≥ 2ϕ
	≥ 25 mm	40 mm + 0,06 x l_v ≥ 2ϕ
Compressed air drilling (CA)	< 25 mm	50 mm + 0,08 x l_v
	≥ 25 mm	60 mm + 0,08 x l_v ≥ 2ϕ

Table B3: Minimum concrete cover c_{min} when using a drilling aid

Drilling method	Rebar diameter Ø	c_{min}
Hammer drilling (HD) Hollow drill bit (HDB)	< 25 mm	30 mm + 0,02 x l_v ≥ 2ϕ
	≥ 25 mm	40 mm + 0,02 x l_v ≥ 2ϕ
Compressed air drilling (CA)	< 25 mm	50 mm + 0,02 x l_v
	≥ 25 mm	60 mm + 0,02 x l_v ≥ 2ϕ



Example of drilling aid

The minimum concrete cover according to EN 1992-1-1:2004+AC:2010 shall be observed.

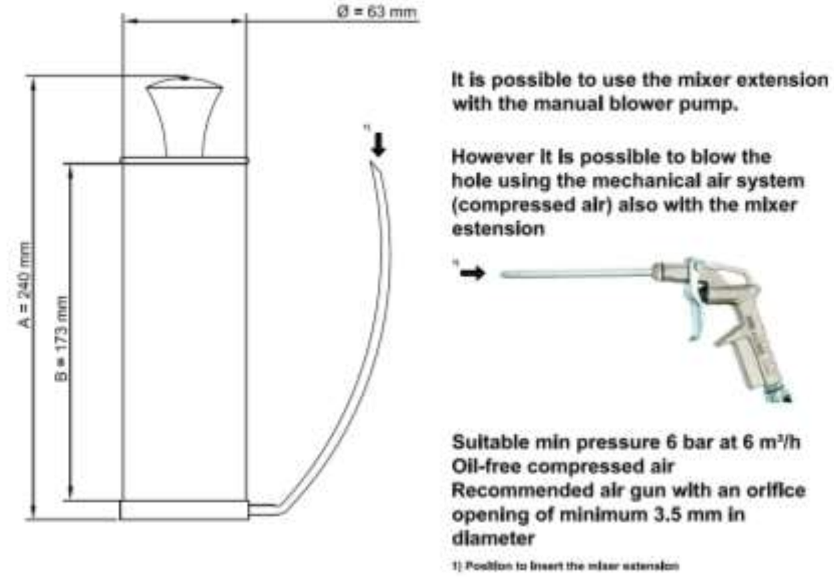
Minimum clear spacing between two post-installed rebar:
a = 40 mm ≥ 4 x Ø

Table B4: Maximum processing time and minimum curing time

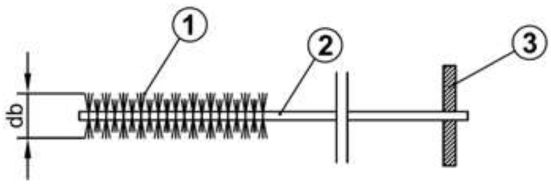
VI 100-PRO (standard version)		
Concrete temperature [C°]	**Maximum processing time [min.]**	**Minimum curing time[1)] [min.]**
-5	65	780
0	45	420
+5	25	90
+10	16	60
+15	11,5	45
+20	7,5	40
+25	5	35
+30	3	30
+35	2	25
+40	1	20
VI 100-PRO-W (version for winter season)		
Concrete temperature [C°]	**Maximum processing time [min.]**	**Minimum curing time[1)] [min.]**
-5	40	210
0	25	100
+5	15	70
+10	10	50
+15	7	35
+20	5	30
VI 100-PRO-T (version for summer season)		
Concrete temperature [C°]	**Maximum processing time [min.]**	**Minimum curing time[1)] [min.]**
+20	14	60
+25	11	50
+30	8	40
+35	6	30
+40	4	20

[1)] The minimum time from the end of the mixing to the time when the rebar may be loaded. Minimum mortar temperature for installation +5°C. Maximum mortar temperature for installation +30°C. For wet condition the curing time must be double.

Manual Blower pump: nominal dimensions



Mixer extension (from 380 mm to 1000 mm) with nominal diameter 8 or 10 mm

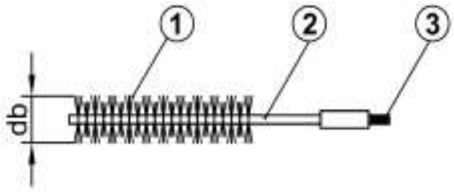


① Steel bristles
② Steel stem
③ Wood handle

Table B5: Standard brush details (manual brush)

Rebar diameter [mm]		Ø8		Ø10		Ø12		Ø14	Ø16
d_0	Nominal drill hole [mm]	10[1)]	12[1)]	12[1)]	14[1)]	14[1)]	16[1)]	18	20
d_b	Brush diameter [mm]	12	14	14	16	16	18	20	22

[1)] Each of two given values can be used



1. Steel bristles
2. Steel stem
3. Threaded connection for drilling tool extension
4. Extension special brush
5. Drilling tool connection (SDS connection)

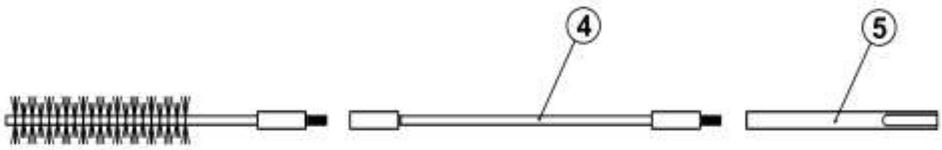


Table B6: Special brush details (mechanical brush)

Rebar diameter [mm]		**Ø8**		**Ø10**		**Ø12**		**Ø14**	**Ø16**	**Ø20**	**Ø22**	**Ø25**	**Ø28**	**Ø30**	**Ø32**
d_0	Nominal drill hole [mm]	10[1)]	12[1)]	12[1)]	14[1)]	14[1)]	16[1)]	18	20	25	26	30	35	35	40
d_b	Brush diameter [mm]	12	14	14	16	16	18	20	22	27	27	32	37	37	42

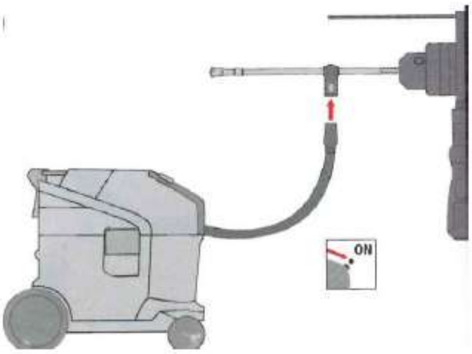
1) Each of two given values can be used

Installation with hollow drill bit (HDB)

This drilling method is a hammer drilling method.

This drilling system removes the dust and cleans the bore hole during the drilling operation when used in accordance with the user's manual.

This drilling system include a vacuum cleaner. A suitable dust extraction system must be used.
e.g. Bosch GAS 35 M AFC or a comparable dust extraction system with equivalent performance data.



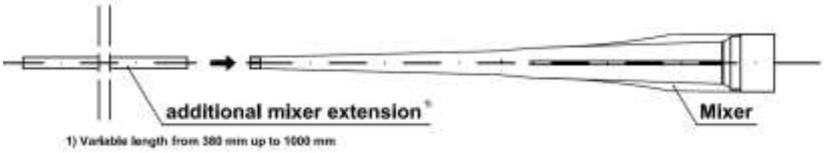
Switch-on the vacuum cleaner before to drill

Table B7: HDB installation diameters

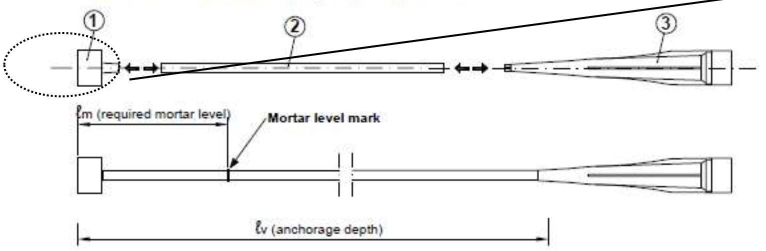
Rebar diameter [mm]		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø28	Ø30
d_0	Nominal drill hole [mm]	10 1) 12 1)	12 1) 14 1)	14 1) 16 1)	18	20	25	30	35	35

1) Each of two given values can be used

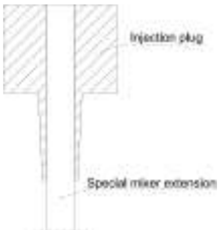
Use the mixer extension (assembled on the standard mixer) for the injection up to 300 mm if necessary.



Use this system for special conditions.



Insert the special mixer extension in the inner diameter of the injection plug up to reach the top of the plug

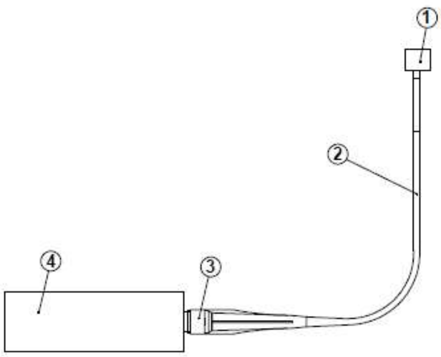


① Injection plug (nominal diameter according to the nominal diameter of drilled hole)
② Special mixer extension (variable length with external diameter 10 mm)
Mark the required mortar level ℓ_m and embedment depth ℓ_v with tape or marker on the injection extension. Quick estimation: $\ell_m = 1/3 \cdot \ell_v$
Continue injection until the mortar level mark ℓ_m becomes visible.
③ Mixer (suitable for all size of cartridge)

These tools allow the application in special conditions:
- installation with anchorage depth greater than 300 mm;
- overhead installation.
For these applications it is recommanded the use of pneumatic or battery dispenser.

System assembled

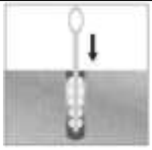
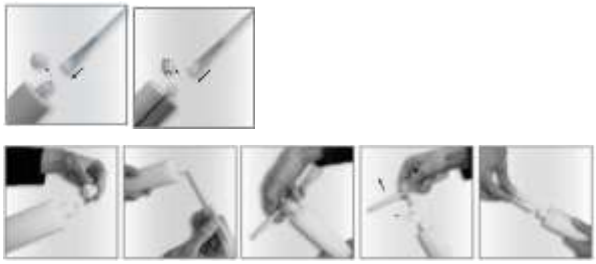
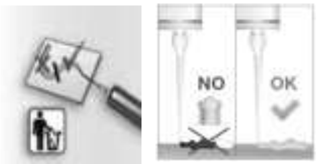
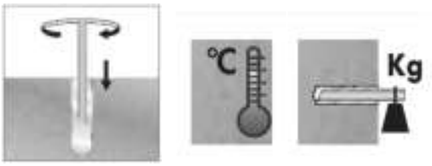
① Injection plug
② Special mixer extension
③ Mixer
④ Cartridge
⑤ Sample of dispenser

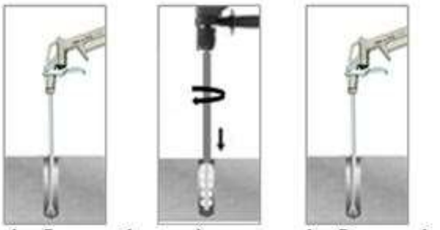
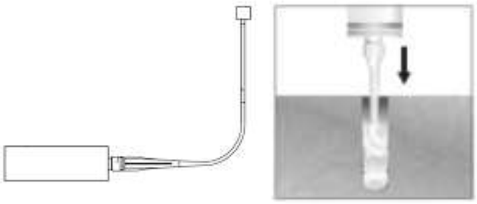


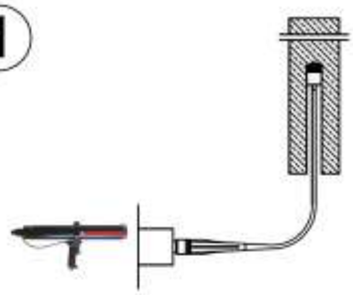
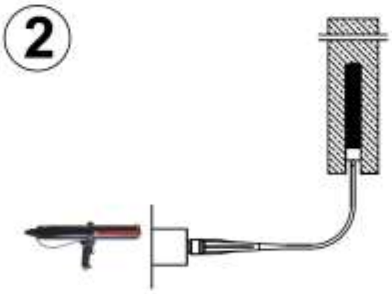
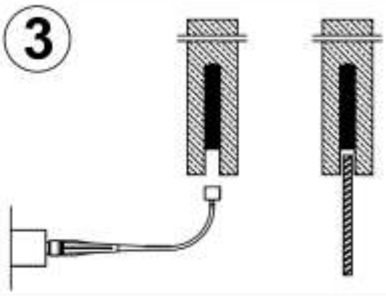
Injection system VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T for rebar connections	Annex B8 of European Technical Assessment ETA-14/0120
Product description Tools for installation (1)	

Table B8: Mortar injection pumps

Pumps (injection dispensers)	Cartridges	Clean hole tools	Depth of the drill hole
 *Manual*	420 ml 400 ml 380 ml	Blower pump or compressed air and standard brush or special brush or HDB	to 300 mm
 *Manual*	345 ml 300 ml 280 ml 165 ml	Blower pump or compressed air and standard brush or special brush or HDB	to 300 mm
 *Manual*	300 ml 280 ml 165 ml	Blower pump or compressed air and standard brush or special brush or HDB	to 300 mm
 *Pneumatic*	825 ml	Compressed air and special brush or HDB	300 mm to 1000 mm*
 *Pneumatic*	420 ml 400 ml 380 ml	Compressed air and special brush or HDB	300 mm to 1000 mm*
 *Battery*	420 ml 400 ml 380 ml 345 ml	Compressed air and special brush or HDB	300 mm to 1000 mm*
* Note: use the mixer extension described in Annex B8 for the injection of the mortar			

1		Drill the hole with the correct diameter and depth using a rotary percussive machine. Check the perpendicularity of the hole during the drilling operation. **In case of use of hollow drill bit (Annex B7) proceed directly to the point 3.**
2	 **4x blower manual pump**  **4x standard brush**  **4x blower manual pump** **if necessary use a mixer extension for the blower operation (see Annex B8)**	Clean the hole from drilling dust: the hole shall be cleaned by at least 4 blowing operations, by at least 4 brushing operations followed again by at least 4 blowing operations; before brushing clean the brush and check (see Annex B5, standard brush) if the brush diameter is sufficient. For the blower tools see Annex B5.
3		For coaxial, peeler and side by side cartridges unscrew the front cup, screw on the mixer and insert the cartridge into the gun. For CIC sizes, unscrew the front cup, pull-out the steel closing clip according to the following operation: 1) Insert the mixer in the eye of the plastic extractor; 2) Pull the extractor to unhook the steel closing clip of the foil. In the version without the extractor cut the foil pack. After that screw on the mixer and insert the cartridge in the gun.
4		Before starting to use the cartridge, eject a first part of the product, being sure that the two components are completely mixed. The complete mixing is reached only after that the product, obtained by mixing the two components, comes out from the mixer with a uniform colour.
5	 **if necessary, use a mixer extension for the injection (see Annex B8)**	Fill the drilled hole uniformly starting from the drilled hole bottom, in order to avoid entrapment of the air; remove the mixer slowly bit by bit during pressing-out; filling the drill hole with a quantity of the injection mortar corresponding to 2/3 of the drill hole depth.
6	 **ATTENTION** **Use rebars dry and free oil and other contaminants**	Insert immediately the rebar, marked according to the proper anchorage depth, slowly and with a slight twisting motion, removing excess of injection mortar around the rebar. Observe the processing time according Annex B4. Wait the curing time according Annex B4.

1	See point 1 Annex B10. In case of use of hollow drill bit (HDB) proceed directly to the point 3.	
2	 ATTENTION: compressed air free oil	Clean the hole from drilling dust: the hole shall be cleaned by at least 4 blowing operations (5 seconds for single operation) with compressed air, by at least 4 brushing operations with special brush followed again by at least 4 blowing operations (5 seconds for single operation) with compressed air. Before brushing clean the brush and check if the brush diameter is sufficient (see Annex B6).
3	See point 3 Annex B10.	
4	See point 4 Annex B10.	
5		Before starting the injection, assemble the system according to Annex B8. After that, fill the drilled hole uniformly from the drilled hole bottom, in order to avoid entrapment of the air; remove the special mixer extension with injection plug slowly bit by bit during pressing-out; filling the drill hole with a quantity of the injection mortar corresponding to 2/3 of the drill hole depth.
6	See point 6 Annex B10.	

1 	**1 - Start injection** **Inject from the bottom of the hole. Maintain this position during the injection phase.**
2 	**2 - Injection phase** **Inject the product about 2/3 of the hole depth. During the injection maintain this position to assure the correct installation**
3 	**3 - End injection** **Remove the injection plug. Insert immediately the rebar (turn the rebar during the insertion).**
4 	**4 - End installation** **To avoid the slipping of the rebar during the open time of the product (due to the rebar own weigth) use a temporary interlocking element (for ex. wedge of wood)**

Minimum anchorage length and minimum lap length under static loading

The minimum anchorage length $l_{b,min}$ and the minimum lap length $l_{0,min}$ according to EN 1992-1-1:2004+AC:2010 shall be multiplied by the relevant amplification factor $\alpha_{lb,50y} = \alpha_{lb,100y}$ given in Table C1.
The design bond strength $f_{bd,PIR,50y} = f_{bd,PIR,100y}$ is given in Table C3. It is obtained by multiplying the bond strength f_{bd} according to EN 1992-1-1:2004+AC:2010 with the factor $k_{b,50y} = k_{b,100y}$ according to Table C2.

Table C1: Amplification factor $\alpha_{lb,50y} = \alpha_{lb,100y}$ related to the concrete class and drilling method

Concrete class	Drilling method	Bar size	Amplification factor $\alpha_{lb,50y} = \alpha_{lb,100y}$
C12/15 to C50/60	Hammer drilling (HD), hollow drill bit (HDB) and compressed air drill (CA)	8 mm to 32 mm	1,0

Table C2: Bond efficiency factor $k_{b,50y} = k_{b,100y}$ related to concrete class and drilling method for a working life of 50 and 100 years

$k_{b,50y} = k_{b,100y}$ for perforation with hammer drill (HD), hollow drill bit (HDB) and compressed air drill (CA)	Concrete class								
	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Ø8 to Ø14	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
Ø16 to Ø20	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	0,93
Ø22	1,00	1,00	1,00	1,00	1,00	1,00	1,00	0,92	0,93
Ø24 to Ø25	1,00	1,00	1,00	1,00	1,00	1,00	1,00	0,92	0,86
Ø28	1,00	1,00	1,00	1,00	1,00	1,00	0,91	0,84	0,79
Ø30 to Ø32	1,00	1,00	1,00	1,00	0,89	0,80	0,73	0,67	0,63

Table C3. Design values of $f_{bd,PIR,50y}$ [1)] = $f_{bd,PIR,100y}$ according to EN 1992-1-1:2004+AC:2010 for hammer drilling (HD), hollow drill bit (HDB) and compressed air drill (CA) for a working life of 50 and 100 years

Rebar diameter [mm]	Design values of $f_{bd,PIR,50y} = f_{bd,PIR,100y}$ [N/mm²]								
	C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Ø8 to Ø14	1,60	2,00	2,30	2,70	3,00	3,40	3,70	4,00	4,30
Ø16 to Ø20	1,60	2,00	2,30	2,70	3,00	3,40	3,70	4,00	4,00
Ø22	1,60	2,00	2,30	2,70	3,00	3,40	3,70	3,70	4,00
Ø24 to Ø25	1,60	2,00	2,30	2,70	3,00	3,40	3,70	3,70	3,70
Ø28	1,60	2,00	2,30	2,70	3,00	3,40	3,40	3,40	3,40
Ø30 to Ø32	1,60	2,00	2,30	2,70	2,70	2,70	2,70	2,70	2,70

[1)] The values given are valid for good bond condition according to EN 1992-1-1:2004+AC:2010.
For all other bond conditions multiply the values by 0,7.

Minimum anchor length and minimum lap length under seismic loading

The minimum anchorage length $l_{b,min}$ and the minimum lap length $l_{0,min}$ according to EN 1992-1-1:2004+AC:2010 shall be multiplied by the relevant amplification factor $\alpha_{lb,seis,50y} = \alpha_{lb,seis,100y}$ given in Table C1.
The design bond strength $f_{bd,seis,50y} = f_{bd,seis,100y}$ is given in Table C5. It is obtained by multiplying the bond strength $f_{bd,PIR}$ according to EN 1992-1-1:2004+AC:2010 with the factor $k_{b,seis,50y} = k_{b,seis,100y}$ according to Table C4. The minimum concrete cover according to Annex B3 and $c_{min,seis}$ = 2 Ø applies.

Table C4: Amplification factor $\alpha_{lb,seis,50y} = \alpha_{lb,seis,100y}$ related to the concrete class for a working life of 50 and 100 years

Concrete class	Drilling method	Bar size	Amplification factor $\alpha_{lb,seis,50y} = \alpha_{lb,seis,100y}$
C16/20 to C50/60	All drilling method	12 mm to 32 mm	1,0

Table C5: Bond efficiency factor $k_{b,seis,50y} = k_{b,seis,100y}$ related to concrete class and drilling method for a working life of 50 and 100 years

$k_{b,seis,50y} = k_{b,seis,100y}$ for perforation with hammer drill (HD), hollow drill bit (HDB) and compressed air drill (CA)	Concrete class							
	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Ø12 to Ø25	1,00	1,00	0,85	0,77	0,68	0,62	0,58	0,53
Ø28 to Ø32	1,00	0,87	0,74	0,67	0,59	0,54	0,50	0,47

Table C6: Design values of $f_{bd,PIR,seis,50y}$[1)] = $f_{bd,PIR,seis,100y}$ for hammer drilling (HD), hollow drill bit (HDB) and compressed air drill (CA) for a working life of 50 and 100 years

Rebar diameter [mm]	Design values of $f_{bd,PIR,seis,50y} = f_{bd,PIR,seis,100y}$ [N/mm²]							
	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Ø12 to Ø25	2,00	2,30	2,30	2,30	2,30	2,30	2,30	2,30
Ø28 to Ø32	2,00	2,00	2,00	2,00	2,00	2,00	2,00	2,00

[1)] The values given are valid for good bond condition according to EN 1992-1-1:2004+AC:2010.
For all other bond conditions multiply the values by 0,7.

Design value of the bond strength $f_{bd,fi,50y} = f_{bd,fi,100y}$ under fire exposure for concrete classes C12/15 to C50/60 (all drilling methods):

The design value of the bond strength $f_{bd,fi}$ under fire exposure has to be calculated by the following equation:

$$f_{bd,fi}(\theta) = k_{fi}(\theta) \cdot f_{bd,PIR} \cdot \frac{\gamma_c}{\gamma_{M,fi}}$$

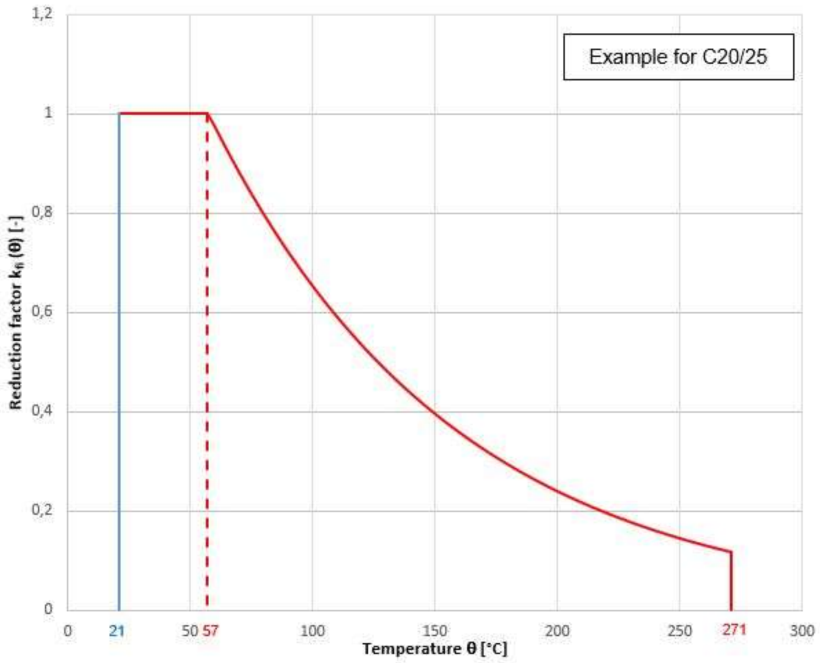
If 21°C ≤ θ ≤ 271°C: $\boldsymbol{k_{fi}(\theta) = \frac{17{,}563 \cdot e^{-0.01\theta}}{f_{bd,PIR} \cdot 4{,}3} \leq 1{,}0}$

If θ > 271°C: $k_{fi}(\theta) = 0$

$f_{bd,fi}(\theta)$ = Design value of the bond strength in case of fire exposure in N/mm²
(θ) = Temperature in °C in the mortar layer
$k_{fi}(\theta)$ = Reduction factor under fire exposure
$f_{bd,PIR}$ = Design value of the bond strength in N/mm², according to Table C3 considering the concrete class, the rebar diameter, the drilling method and the bond conditions according to EN 1992-1-1:2004+AC:2010
γ_c = Partial safety factor according to EN 1992-1-1:2004+AC:2010
$\gamma_{M,fi}$ = Partial safety factor according to EN 1992-1-2:2004+AC:2008

For evidence under fire exposure the anchorage length shall be calculated according to EN 1992-1-1:2004+AC:2010, Equation 8.3 using the temperature-dependent ultimate bond strength $f_{bd,fi}$.

Figure C1: Example graph of reduction factor $k_{fi}(\theta)_{,50y} = k_{fi}(\theta)_{,100y}$ for concrete classes C20/25 for good bond conditions:



Injection system VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T for rebar connections	Annex C3 of European Technical Assessment ETA-14/0120
Performances Design values of $f_{bd,fi}(\theta)_{,50y} = f_{bd,fi}(\theta)_{,100y}$ under fire exposure with reduction factor $k_{fi}(\theta)_{,50y} = k_{fi}(\theta)_{,100y}$	

INSTYTUT TECHNIKI BUDOWLANEJ

Designated according to Article 29 of Regulation (EU) No 305/2011 and member of EOTA (European Organisation for Technical Assessment)

Member of EOTA
www.eota.eu

European Technical Assessment

ETA-14/0119 of 08/08/2024

General Part

Technical Assessment Body issuing the European Technical Assessment	Instytut Techniki Budowlanej
Trade name of the construction product	VI 100-PRO VI 100-PRO-W VI 100-PRO-T
Product family to which the construction product belongs	Bonded fasteners for use in concrete
Manufacturer	ALSAFIX S.A.S. 114a rue Principale 67240 Gries France
Manufacturing plant	ALSAFIX S.A.S. Manufacturing plant 1
This European Technical Assessment contains	32 pages including 3 Annexes which form an integral part of this Assessment
This European Technical Assessment is issued in accordance with regulation (EU) No 305/2011, on the basis of	European Assessment Document (EAD) 330499-02-0601 ”Bonded fasteners for use in concrete”
This version replaces	ETA-14/0119 issued on 07/10/2019

Specific Part

1 Technical description of the product

VI 100-PRO, VI 100-PRO-W and VI 100-PRO-T are bonded fasteners (injection type) consisting of an injection mortar cartridge using an applicator gun equipped with a special mixing nozzle and steel element: commercial threaded rod of the sizes M8 to M30 with hexagon nut and washer or reinforcing bar (rebar) Ø8 to Ø32 mm.

The steel element is placed into a drilled hole previously injected (using an applicator gun) with a mortar with a slow and slight twisting motion. The steel element is anchored by the bond between steel element, mortar and concrete.

An illustration and the description of the products are given in Annex A.

2 Specification of the intended use in accordance with the applicable European Assessment Document (EAD)

The performances given in clause 3 are only valid if the fasteners are used in compliance with the specifications and conditions given in Annex B.

The provisions made in this European Technical Assessment are based on an assumed working life of the fastener of 50 and/or 100 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

3.1 Performance of the product

3.1.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance to tension load and shear load (static and quasi static loading), displacements	See Annex C1 to C7
Characteristic resistance for seismic performance category C1	See Annex C8
Characteristic resistance for seismic performance category C2	See Annex C9

3.1.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Class A1
Resistance to fire	See Annex C10 to C12

3.2 Methods used for the assessment

The assessment has been made in accordance with EAD 330499-02-0601.

4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

According to Decision 96/582/EC of the European Commission the system 1 of assessment and verification of constancy of performance applies (see Annex V to regulation (EU) No 305/2011).

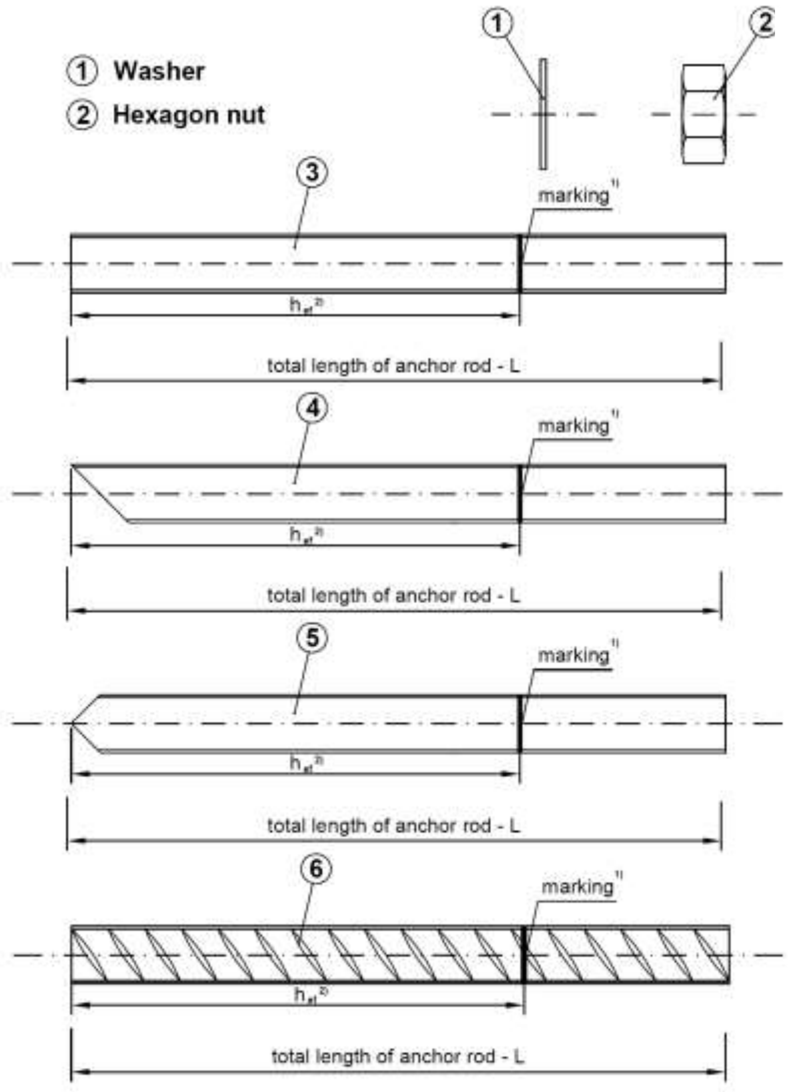
5 Technical details necessary for the implementation of the AVCP system, as provided in the applicable European Assessment Document (EAD)

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited in Instytut Techniki Budowlanej.

For type testing the results of the tests performed as part of the assessment for the European Technical Assessment shall be used unless there are changes in the production line or plant. In such cases the necessary type testing has to be agreed between Instytut Techniki Budowlanej and the notified body.

Issued in Warsaw on 08/08/2024 by Instytut Techniki Budowlanej

Anna Panek, MSc
Deputy Director of ITB



Filling washer
(to fill annular gap)

3) Version 1 – rod with flat end with marking on h_{ef}

4) Version 2 – rod with 45° cutted end with marking on h_{ef}

5) Version 3 – rod with V shape end with marking on h_{ef}

6) Rebar – ribbed reinforcing bar with marking on h_{ef}

1) Marking according to clause 1.1 of EAD 330499-02-0601
2) Effective anchorage depth according to Table B1 and B2 (Annex B2 and B3)

VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex A1** of European Technical Assessment ETA-14/0119
Product description Steel elements	

Table A1: Threaded rods

Designation	Property class	Characteristic steel ultimate strength	Characteristic steel yield strength	Fracture elongation	Material
Steel, zinc plated **electroplated ≥ 5 µm acc. to EN ISO 4042** **hot-dip galvanized ≥ 40 µm acc. to EN ISO 1461**					
Threaded rod	4.8	$f_{uk} \geq 400$ N/mm²	$f_{yk} \geq 320$ N/mm²	$A_5 > 8\%$[1)]	EN ISO 898-1
	5.8	$f_{uk} \geq 500$ N/mm²	$f_{yk} \geq 400$ N/mm²	$A_5 > 8\%$[1)]	
	8.8	$f_{uk} \geq 800$ N/mm²	$f_{yk} \geq 640$ N/mm²	$A_5 \geq 12\%$[1)]	
	10.9	$f_{uk} \geq 1000$ N/mm²	$f_{yk} \geq 900$ N/mm²	$A_5 > 9\%$[1)]	
Hexagon nut	4	for class 4.8 rods			EN 898-2
	5	for class 5.8 rods			
	8	for class 8.8 rods			
	10	for class 10.9 rods			
Washer	Steel according to EN ISO 7089; corresponding to anchor rod material				
Stainless steel A2 **Stainless steel A4** **High corrosion resistance stainless steel (HCR)**	**(Materials) 1.4301, 1.4307, 1.4567, 1.4541** **(Materials) 1.4401, 1.4404, 1.4571, 1.4362, 1.4578** **(Materials) 1.4529, 1.4565**				
Threaded rod	**Property class**	**Characteristic steel ultimate strength**	**Characteristic steel yield strength**	**Fracture elongation**	EN 10088 EN ISO 3506
	50	$f_{uk} \geq 500$ N/mm²	$f_{yk} \geq 210$ N/mm²	$A_5 > 8\%$[1)]	
	70	$f_{uk} \geq 700$ N/mm²	$f_{yk} \geq 450$ N/mm²	$A_5 \geq 12\%$[1)]	
	80	$f_{uk} \geq 800$ N/mm²	$f_{yk} \geq 600$ N/mm²	$A_5 \geq 12\%$[1)]	
Hexagon nut	50	for class 50 rods			EN 10088 EN ISO 3506
	70	for class 70 rods			
	80	for class 80 rods			
Washer	Steel according to EN 10088; corresponding to anchor rod material				

1) For seismic performance category C1 and C2, $A_5 > 19\%$

Commercial standard threaded rods may be used, with:

- material and mechanical properties according to Table A1,
- confirmation of material and mechanical properties by inspection certificate 3.1 according to EN 10204:2004,
- marking of the threaded rod with the embedment depth.

Note: Commercial standard threaded rods made of galvanized steel with property class above 8.8 are not permitted in some Member States.

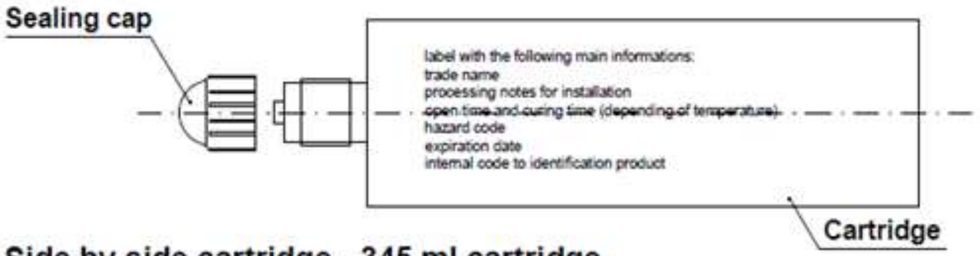
Table A2: Reinforcing bars (Rebar)

Designation	Material
Rebar according to EN 1992-1-1:2004+AC:2010	Bars and de-coiled rods Class B or C with f_{yk} and k according to EN 1992-1-1:2004+AC:2010 $f_{uk} = f_{tk} = k \cdot f_{yk}$ Rib height of the bar (h) in the range 0,05d ≤ h ≤ 0,07d

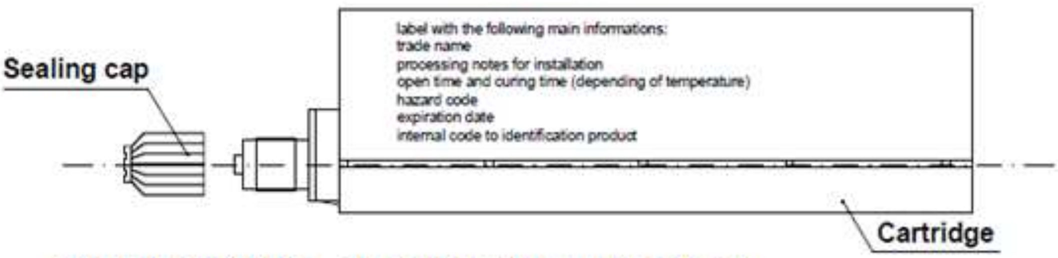
Table A3: Injection mortars

Product	Composition
VI 100-PRO VI 100-PRO-W VI 100-PRO-T (two component injection mortars)	Additive: quartz Bonding agent: vinyl ester resin styrene free Hardener: dibenzoyl peroxide

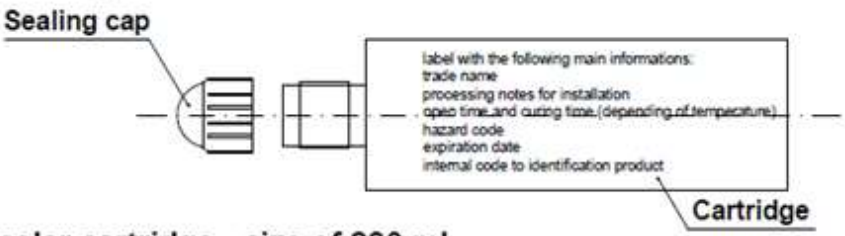
Coaxial cartridge - sizes from 75 ml to 420 ml



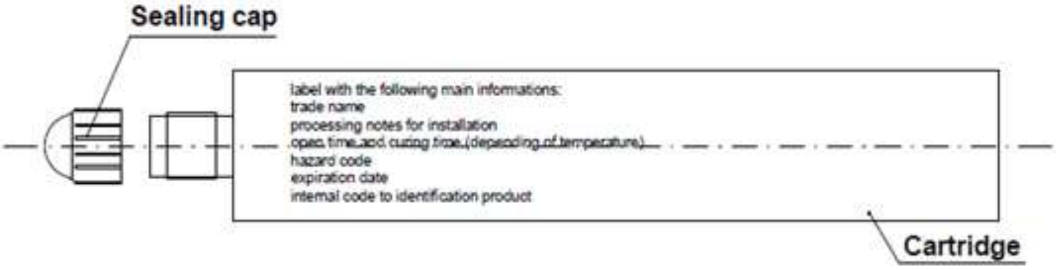
Side by side cartridge - 345 ml cartridge



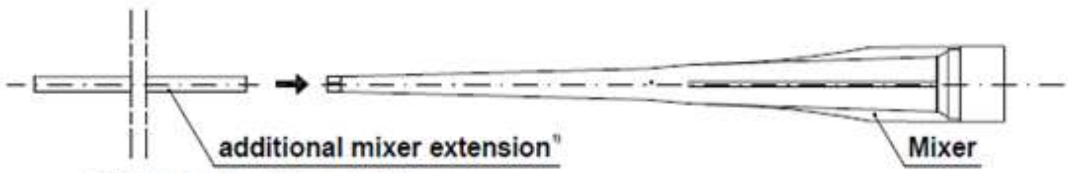
CIC foil cartridge - sizes from 165 ml to 300 ml



Coaxial peeler cartridge - size of 280 ml



MIXER - the mixer is suitable for each type of cartridge



1) Variable length from 380 mm up to 1000 mm

VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex A4** of European Technical Assessment ETA-14/0119
Product description Cartridge types and sizes	

Specifications of intended use

Anchors subject to:

- Static and quasi-static loads: sizes from M8 to M30 and from Ø8 to Ø32.
- Seismic performance category C1: sizes from M12 to M20, rods with $f_{uk} \leq 800$ N/mm^2 and fracture elongation $A_5 \geq 19\%$.
- Seismic performance category C2: sizes M12 and M16, rods with $f_{uk} \leq 800$ N/mm^2 and fracture elongation $A_5 \geq 19\%$.
- Fire exposure: sizes from M10 to M20, steel class 5.8 to 8.8 and stainless steel A4.

Working life:

Working life of the bonded fasteners of 50 and/or 100 years.

Base material:

- Reinforced or unreinforced normal weight concrete of strength class C20/25 at minimum to C50/60 at maximum according to EN 206.
- Uncracked concrete: sizes from M8 to M30 and from Ø8 to Ø32.
- Cracked concrete: sizes from M10 to M20.

Temperature range:

The anchors may be used in the following temperature range:

- -40°C to +40°C (max. short term temperature +40°C and max. long term temperature +24°C).
- -40°C to +80°C (max. short term temperature +80°C and max. long term temperature +50°C).
- -40°C to +120°C (max. short term temperature +120°C and max. long term temperature +72°C).

Use conditions (environmental conditions):

- Structures subject to dry internal conditions: all materials according to Table A1 and A2.
- For all other conditions according to EN 1993-1-4:2006+A1:2015 corresponding to corrosion resistance class:
  - stainless steel A2 according to Annex A2, Table A1 - CRC II,
  - stainless steel A4 according to Annex A2, Table A1 - CRC III,
  - high corrosion resistance steel (HCR) according to Annex A2, Table A1 - CRC V.

Installation:

- Dry or wet concrete (use category I1): sizes from M8 to M30 and from Ø8 to Ø32.
- Flooded holes with the exception of seawater (use category I2): sizes from M8 to M30 and from Ø8 to Ø32.
- Installation direction D3 (downward and horizontal and upwards installation): sizes from M8 to M30 and from Ø8 to Ø32.
- The anchors are suitable for hammer drilled holes (HD), for hollow drill bit (HDB) and for compressed air drill (CA): sizes from M8 to M30 and from Ø8 to Ø32.

Design methods:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.).
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages under static or quasi-static loads are designed according to EN 1992-4 and EOTA Technical Report TR 055.
- Anchorages under seismic actions are designed according to EN 1992-4.
- Anchorages under fire exposure are designed according to EOTA Technical Report TR 082.

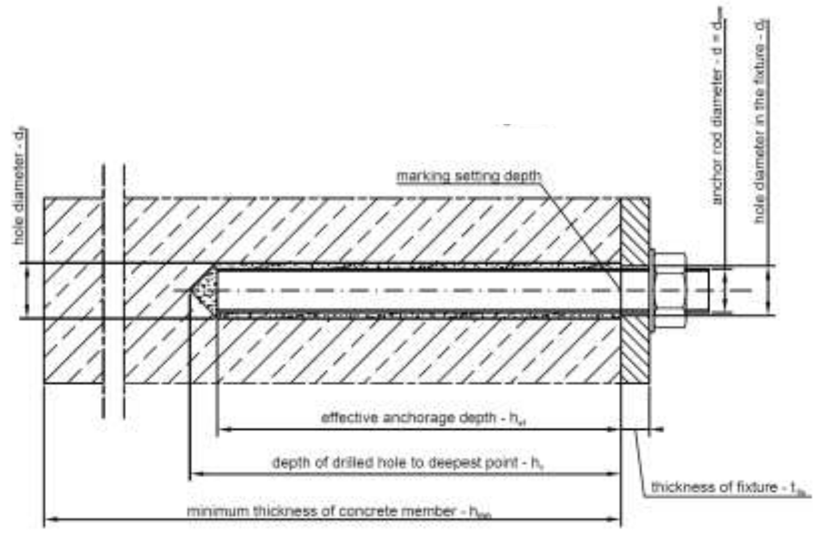


Table B1: Installation data for threaded rods

Size		M8	M10	M12	M16	M20	M24	M27	M30
Nominal drilling diameter	d_0 [mm]	10	12	14	18	22[1)] 24[1)]	28	30	35
Maximum diameter hole in the fixture	d_{fix} [mm]	9	12	14	18	22	26	30	33
Effective embedment depth	$h_{ef,min}$ [mm]	60	70	80	100	120	145	145	145
	$h_{ef,max}$ [mm]	160	200	240	320	400	480	540	600
Depth of the drilling hole	h_1 [mm]	h_{ef} + 5 mm							
Minimum thickness of the concrete slab	h_{min} [mm]	h_{ef} + 30 mm; ≥ 100 mm			$h_{ef} + 2d_0$				
Maximum setting torque moment	T_{fix} [N·m]	10	20	40	80	130	200	250	280
Thickness to be fixed	$t_{fix,min}$ [mm]	> 0							
	$t_{fix,max}$ [mm]	< 1500							
Minimum spacing	s_{min} [mm]	40	50	60	75	90	115	120	140
Minimum edge distance	c_{min} [mm]	35	40	45	50	55	60	75	80

1) Each of two given values can be used

VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex B2** of European Technical Assessment ETA-14/0119
Intended use Installation data for threaded rods	

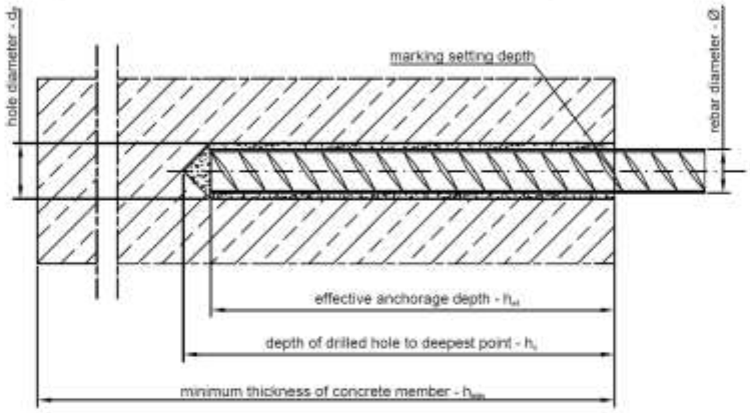


Table B2: Installation data for rebars

Size		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø28	Ø32
Nominal drilling diameter	d_0 [mm]	10[1)] 12[1)]	12[1)] 14[1)]	14[1)] 16[1)]	18	20	25	30	35	40
Effective embedment depth	$h_{ef,min}$ [mm]	60	70	80	80	100	120	150	180	200
	$h_{ef,max}$ [mm]	160	200	240	280	320	400	500	560	640
Depth of the drilling hole	h_1 [mm]	h_{ef} + 5 mm								
Minimum thickness of the concrete slab	h_{min} [mm]	h_{ef} + 30 mm; ≥ 100 mm		$h_{ef} + 2d_0$						
Minimum spacing	s_{min} [mm]	40	50	60	75	75	90	115	120	140
Minimum edge distance	c_{min} [mm]	35	40	45	50	50	55	60	75	80

[1)] Each of two given values can be used

Table B3: Maximum processing time and minimum curing time

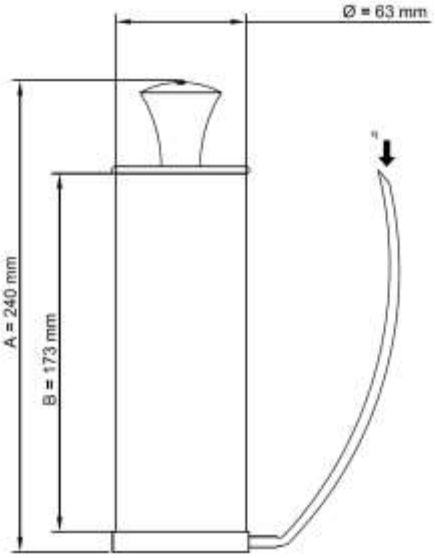
VI 100-PRO		
Concrete temperature [C°]	**Maximum processing time [min.]**	**Minimum curing time [1) [min.]**
-10	105	1440
-5	65	840
0	45	420
+5	25	90
+10	16	60
+15	11,5	45
+20	7,5	40
+25	5	35
+30	3	30
+35	2	25
+40	1	20
VI 100-PRO-W		
Concrete temperature [C°]	**Maximum processing time [min.]**	**Minimum curing time [1) [min.]**
-20	120	2880
-15	90	1500
-10	60	900
-5	40	210
0	25	100
+5	15	70
+10	10	50
+15	7	35
+20	5	30
VI 100-PRO-T		
Concrete temperature [C°]	**Maximum processing time [min.]**	**Minimum curing time [1) [min.]**
+20	14	60
+25	11	50
+30	8	40
+35	6	30
+40	4	20

[1)] The minimum time from the end of the mixing to the time when the anchor may be torque or loaded (whichever is longer). Cartridge temperature from +5°C to +30°C. Minimum cartridge temperature of +15°C for application where the concrete temperature is below 0°C.

For wet condition and flooded holes, the curing time must be double.

VI 100-PRO VI 100-PRO-W VI 100-PRO-T	**Annex B4** of European Technical Assessment ETA-14/0119
Intended use Maximum processing time and minimum curing time	

Manual Blower pump: nominal dimensions



It is possible to use the mixer extensior with the manual blower pump.

However it is possible to blow the hole using the mechanical air system (compressed air) also with the mixer estension

Suitable min pressure 6 bar at 6 m³/h
Oil-free compressed air
Recommended air gun with an orifice opening of minimum 3.5 mm in diameter

1) Position to insert the mixer extension

Mixer extension (from 380 mm to 1000 mm) with nominal diameter 10 mm

VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex B5** of European Technical Assessment ETA-14/0119
Intended use Cleaning tools (1)	

Table B4: Standard brush diameter for threaded rods

Threaded rod diameter		M8	M10	M12	M16	M20	M24	M27	M30
d_0	Nominal drill hole [mm]	10	12	14	18	24	28	30	35
d_b	Brush diameter [mm]	12	14	16	20	26	30	35	37

Table B5: Standard brush diameter for rebar

Rebar diameter		Ø8		Ø10		Ø12		Ø14
d_0	Nominal drill hole [mm]	10[1)]	12[1)]	12[1)]	14[1)]	14[1)]	16[1)]	18
d_b	Brush diameter [mm]	12	14	14	16	16	18	20

[1)] Each of two given values can be used

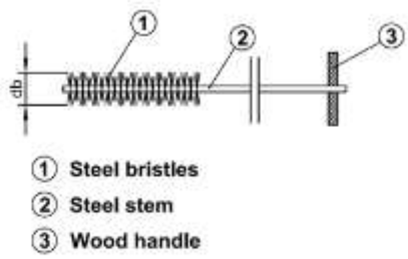
Table B6: Special brush diameter (mechanical brush) for threaded rods

Threaded rod diameter		M16	M20	M24	M27	M30
d_0	Nominal drill hole [mm]	18	24	28	30	35
d_b	Brush diameter [mm]	20	26	30	32	37

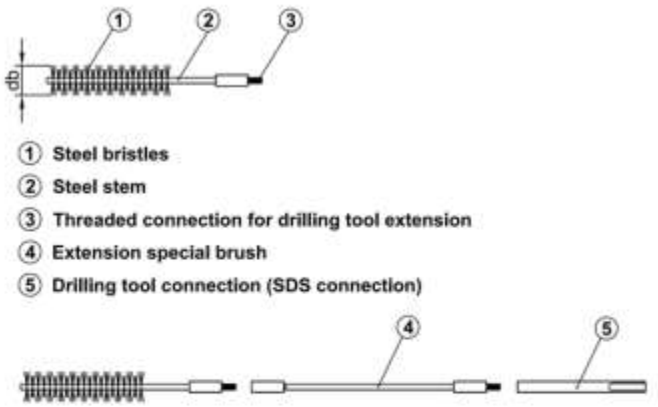
Table B7: Special brush diameter (mechanical brush) for rebar

Threaded rod diameter		Ø8		Ø10		Ø12		Ø14	Ø16	Ø20	Ø25	Ø28	Ø32
d_0	Nominal drill hole [mm]	10[1)]	12[1)]	12[1)]	14[1)]	14[1)]	16[1)]	18	20	25	30	35	40
d_b	Brush diameter [mm]	12	14	14	16	16	18	20	22	27	32	37	42

[1)] Each of two given values can be used



Standard brush



Special (mechanical) brush

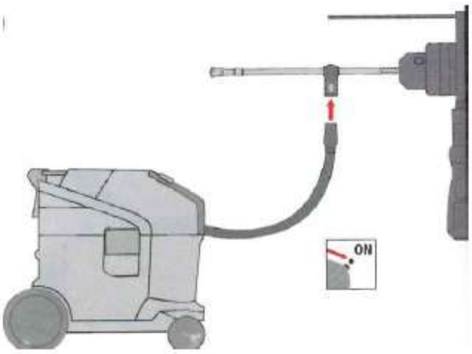
VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex B6** of European Technical Assessment ETA-14/0119
Intended use Cleaning tools (2)	

Hollow Drill Bit (HDB)

This drilling method is a hammer drilling method.

This drilling system removes the dust and cleans the bore hole during the drilling operation when used in accordance with the user's manual.

This drilling system include a vacuum cleaner. A suitable dust extraction system must be used.
e.g. Bosch GAS 35 M AFC or a comparable dust extraction system with equivalent performance data.



Switch-on the vacuum cleaner before to drill

Table B8: HDB perforation diameter for threaded rods

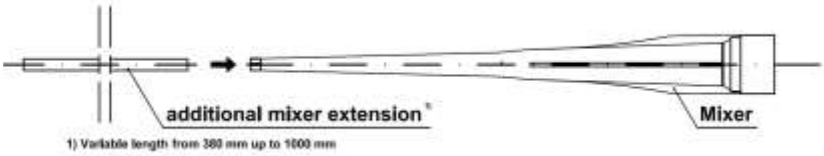
Threaded rod diameter		M8	M10	M12	M16	M20	M24	M27	M30
d_0	Nominal drill hole [mm]	10	12	14	18	24	28	30	35

Table B9: HDB perforation diameter for rebar

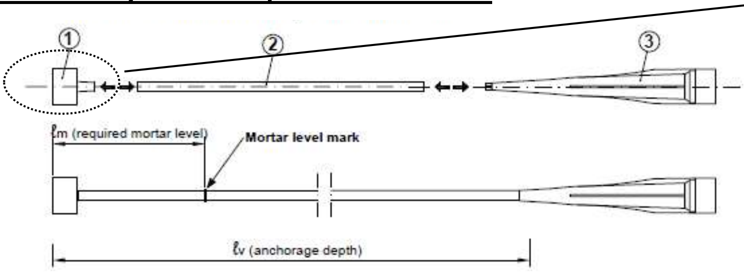
Rebar diameter		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø28
d_0	Nominal drill hole [mm]	10[1)] 12[1)]	12[1)] 14[1)]	14[1)] 16[1)]	18	20	25	30	35

[1)] Each of two given values can be used

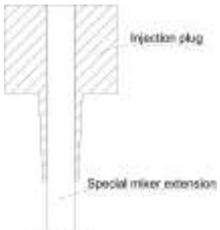
Use the mixer extension (assembled on the standard mixer) for the injection up to 300 mm if necessary.



Use this system for special conditions.



Insert the special mixer extension in the inner diameter of the injection plug up to reach the top of the plug



① Injection plug (nominal diameter according to the nominal diameter of drilled hole)

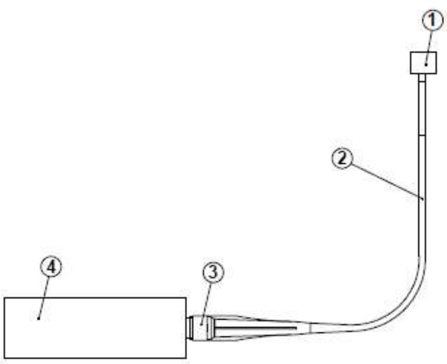
② Special mixer extension (variable length with external diameter 10 mm)
Mark the required mortar level ℓ_m and embedment depth ℓ_v with tape or marker on the injection extension. Quick estimation: $\ell_m = 1/3 \cdot \ell_v$
Continue injection until the mortar level mark ℓ_m becomes visible.

③ Mixer (suitable for all size of cartridge)

These tools allow the application in special conditions:
- installation with anchorage depth greater than 300 mm;
- overhead installation.
For these applications it is recommanded the use of pneumatic or battery dispenser.

System assembled

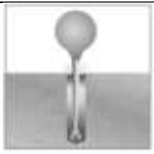
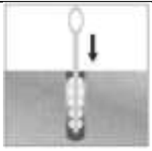
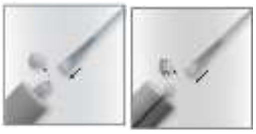
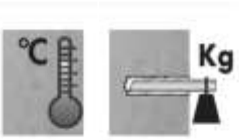
① Injection plug
② Special mixer extension
③ Mixer
④ Cartridge
⑤ Sample of dispenser



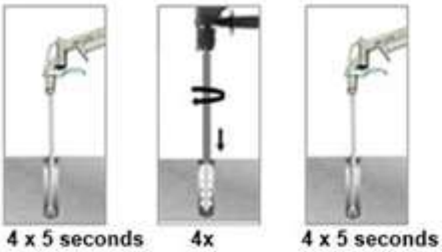
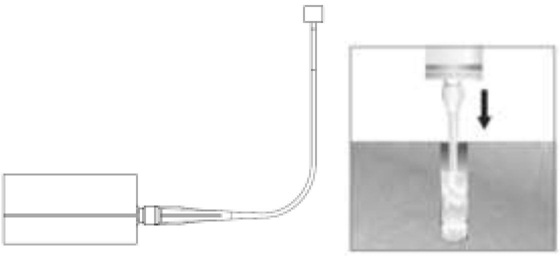
VI 100-PRO VI 100-PRO-W VI 100-PRO-T	**Annex B8** of European Technical Assessment ETA-14/0119
Intended use Tools for installation (1)	

Table B10: Mortar injection pumps

Pumps (injection dispensers)	Cartridges	Types
 *Manual*	420 ml 400 ml 380 ml	Manual (up to 300 mm anchorage depth)
 *Manual*	345 ml 300 ml 280 ml 165 ml	Manual (up to 300 mm anchorage depth)
 *Manual*	300 ml 280 ml 165 ml	Manual (up to 300 mm anchorage depth)
 *Pneumatic*	825 ml	Pneumatic (up to 640 mm anchorage depth)
 *Pneumatic*	420 ml 400 ml 380 ml	Pneumatic (up to 640 mm anchorage depth)
 *Battery*	420 ml 400 ml 380 ml 345 ml 300 ml	Battery (up to 640 mm anchorage depth)

1		Drill the hole with the correct diameter and depth using a rotary percussive machine. Check the perpendicularity of the hole during the drilling operation. **In case of use of hollow drill bit (Annex B7) proceed directly to the clause 3.**
2	   **4x blower manual pump** **4x standard brush** **4x blower manual pump** **if necessary use a mixer extension for the blower operation (see Annex B5)**	Clean the hole from drilling dust: the hole shall be cleaned by at least 4 blowing operations, by at least 4 brushing operations followed again by at least 4 blowing operations; before brushing clean the brush and check (see Annex B6, standard brush) if the brush diameter is sufficient. For the blower tools see Annex B5.
3	 	For coaxial, peeler and side by side cartridges unscrew the front cup, screw on the mixer and insert the cartridge into the gun. For CIC sizes, unscrew the front cup, pull-out the steel closing clip according to the following operation: 1) Insert the mixer in the eye of the plastic extractor; 2) Pull the extractor to unhook the steel closing clip of the foil. In the version without the extractor cut the foil pack. After that screw on the mixer and insert the cartridge in the gun.
4		Before starting to use the cartridge, eject a first part of the product, being sure that the two components are completely mixed. The complete mixing is reached only after that the product, obtained by mixing the two components, comes out from the mixer with a uniform colour.
5	 **if necessary, use a mixer extension for the injection (see Annex B8)**	Fill the drilled hole uniformly starting from the drilled hole bottom, in order to avoid entrapment of the air; remove the mixer slowly bit by bit during pressing-out; filling the drill hole with a quantity of the injection mortar corresponding to 2/3 of the drill hole depth.
6	  **ATTENTION: Steel elements dry and free oil and other contaminants**	Insert immediately the steel element (threaded rod or rebar), marked according to the proper anchorage depth, slowly and with a slight twisting motion, removing excess of injection mortar around the steel element. Observe the processing time according Annex B4. Wait the curing time according Annex B4.

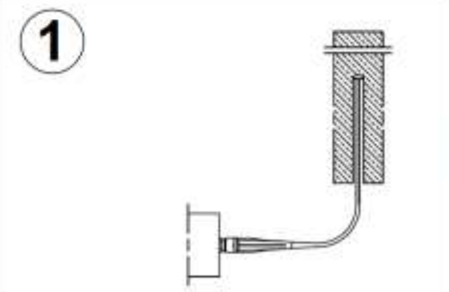
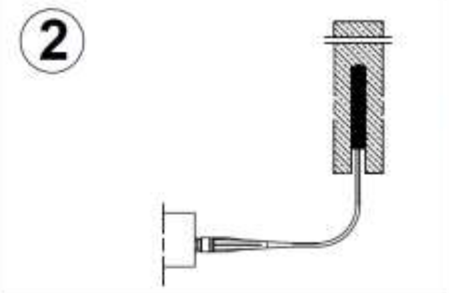
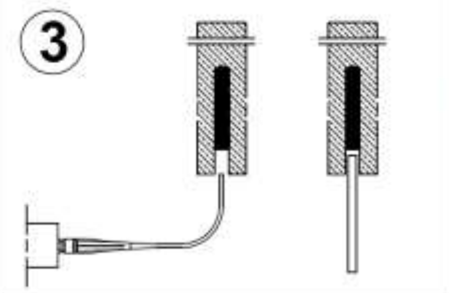
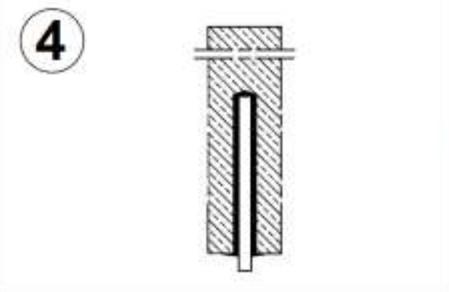
VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex B10** of European Technical Assessment ETA-14/0119
Intended use Installation instruction up to 300 mm depth	

1	**See clause 1 Annex B10.** In case of use of hollow drill bit (HDB) proceed directly to the clause 3.	
2	 ATTENTION: compressed air free oil	Clean the hole from drilling dust: the hole shall be cleaned by at least 4 blowing operations (5 seconds for single operation) with compressed air, by at least 4 brushing operations with special brush followed again by at least 4 blowing operations (5 seconds for single operation) with compressed air. Before brushing clean the brush and check if the brush diameter is sufficient.
3	**See clause 3 Annex B10**	
4	**See clause 4 Annex B10**	
5		Before starting the injection, assemble the system according to Annex B8. After that, fill the drilled hole uniformly from the drilled hole bottom, in order to avoid entrapment of the air; remove the special mixer extension with injection plug slowly bit by bit during pressing-out; filling the drill hole with a quantity of the injection mortar corresponding to 2/3 of the drill hole depth. Procedure for overhead installation are detailed in Annex B12.
6	**See clause 6 Annex B10**	

VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex B11** of European Technical Assessment ETA-14/0119
Intended use Installation instruction up to 640 mm depth	

Overhead installation procedure

In addition to standard procedure, for overhead installation, following the below procedure

	**1 - Start injection** Inject from the bottom of the hole. Use battery or pneumatic dispenser if the anchorage depth is grater than 200 mm.
	**2 - Injection phase** Inject the product about 2/3 of the hole depth. Remove the mixer extension slowly bit by bit during pressing-out.
	**3 - End injection** Remove the mixer extension. Insert immediately the steel element (turn the steel element during the insertion).
	**4 - End installation** To avoid the slipping of the steel element during the open time of the product (due to the steel element own weigth) use a temporary interlocking element (for ex. wedge of wood)

VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex B12** of European Technical Assessment ETA-14/0119
Intended use Overhead installation instruction	

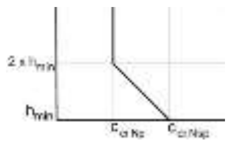
Table C1: Characteristic values for steel tension resistance and steel shear resistance – threaded rods.

Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel failure – characteristic tension resistance										
Steel class 4.8	$N_{Rk,s}$	[kN]	15	23	34	63	98	141	183	224
Steel class 5.8	$N_{Rk,s}$	[kN]	18	29	42	78	122	176	229	280
Steel class 8.8	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	367	449
Steel class 10.9	$N_{Rk,s}$	[kN]	37	58	84	157	245	353	459	561
Stainless steel A2, A4, HCR class 50	$N_{Rk,s}$	[kN]	18	29	42	78	122	176	229	280
Stainless steel A2, A4, HCR class 70	$N_{Rk,s}$	[kN]	26	41	59	110	171	247	321	392
Stainless steel A4, HCR class 80	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	367	449
Steel failure – characteristic tension resistance – partial factor										
Steel class 4.8	$\gamma_{Ms,N}$ 1)	[-]	1,50							
Steel class 5.8	$\gamma_{Ms,N}$ 1)	[-]	1,50							
Steel class 8.8	$\gamma_{Ms,N}$ 1)	[-]	1,50							
Steel class 10.9	$\gamma_{Ms,N}$ 1)	[-]	1,40							
Stainless steel A2, A4, HCR class 50	$\gamma_{Ms,N}$ 1)	[-]	2,86							
Stainless steel A2, A4, HCR class 70	$\gamma_{Ms,N}$ 1)	[-]	1,87							
Stainless steel A4, HCR class 80	$\gamma_{Ms,N}$ 1)	[-]	1,60							
Steel failure – characteristic shear resistance without lever arm										
Steel class 4.8	$V^0_{Rk,s}$	[kN]	7	12	17	31	49	71	92	112
Steel class 5.8	$V^0_{Rk,s}$	[kN]	9	14	21	39	61	88	115	140
Steel class 8.8	$V^0_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Steel class 10.9	$V^0_{Rk,s}$	[kN]	18	29	42	78	122	176	230	280
Stainless steel A2, A4, HCR class 50	$V^0_{Rk,s}$	[kN]	9	14	21	39	61	88	115	140
Stainless steel A2, A4, HCR class 70	$V^0_{Rk,s}$	[kN]	13	20	29	55	86	124	160	196
Stainless steel A4, HCR class 80	$V^0_{Rk,s}$	[kN]	15	23	34	63	98	141	184	224
Steel failure – characteristic shear resistance with lever arm										
Steel class 4.8	$M^0_{Rk,s}$	[Nm]	15	30	52	133	260	449	666	900
Steel class 5.8	$M^0_{Rk,s}$	[Nm]	19	37	65	166	324	561	832	1125
Steel class 8.8	$M^0_{Rk,s}$	[Nm]	30	60	105	266	519	898	1331	1799
Steel class 10.9	$M^0_{Rk,s}$	[Nm]	37	75	131	333	649	1123	1664	2249
Stainless steel A2, A4, HCR class 50	$M^0_{Rk,s}$	[Nm]	19	37	66	166	324	561	832	1124
Stainless steel A2, A4, HCR class 70	$M^0_{Rk,s}$	[Nm]	26	52	92	233	454	786	1165	1574
Stainless steel A4, HCR class 80	$M^0_{Rk,s}$	[Nm]	30	60	105	266	519	898	1331	1799
Steel failure – characteristic shear resistance – partial factor										
Steel class 4.8	$\gamma_{Ms,V}$ 1)	[-]	1,25							
Steel class 5.8	$\gamma_{Ms,V}$ 1)	[-]	1,25							
Steel class 8.8	$\gamma_{Ms,V}$ 1)	[-]	1,25							
Steel class 10.9	$\gamma_{Ms,V}$ 1)	[-]	1,50							
Stainless steel A2, A4, HCR class 50	$\gamma_{Ms,V}$ 1)	[-]	2,38							
Stainless steel A2, A4, HCR class 70	$\gamma_{Ms,V}$ 1)	[-]	1,56							
Stainless steel A4, HCR class 80	$\gamma_{Ms,V}$ 1)	[-]	1,33							

1) In the absence of other national regulation

Fracture elongation threaded rod for seismic category C1 and C2 must be A_5 ≥ 19%.
Steel classes 10.9 are not covered for seismic application.

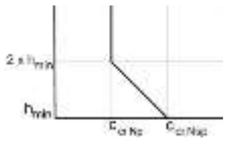
Table C2: Characteristic values for tension resistance in uncracked concrete under static and quasi-static loads – threaded rods. Working life of 50 and 100 years.

Size			M8	M10	M12	M16	M20	M24	M27	M30
Steel failure										
Characteristic resistance	$N_{Rk,s}$	[kN]	See Annex C1 – Table C1							
Partial factor	$\gamma_{Ms,N}$ [1)]	[-]	See Annex C1 – Table C1							
Combined pull-out and concrete cone failure in uncracked concrete C20/25										
Characteristic bond resistance temperature range -40°C / +40°C	$\tau_{Rk,ucr,50}$ $\tau_{Rk,ucr,100}$	[N/mm²]	16,0	12,0	12,0	12,0	9,5	9,5	8,0	8,0
Characteristic bond resistance temperature range -40°C / +80°C	$\tau_{Rk,ucr,50}$ $\tau_{Rk,ucr,100}$	[N/mm²]	11,0	8,5	8,5	8,5	7,0	7,0	6,0	6,0
Characteristic bond resistance temperature range -40°C / +120°C	$\tau_{Rk,ucr,50}$ $\tau_{Rk,ucr,100}$	[N/mm²]	6,0	4,5	4,5	4,5	4,0	4,0	3,0	3,0
Increasing factor	ψ_c	[-]	$(\frac{f_{ck}}{20})^{0.3}$							
Sustained load factor for temperature range -40°C / +40°C	ψ^0_{sus} $\psi^0_{sus,100}$	[-]	0,72							
Sustained load factor for temperature range -40°C / +80°C			0,74							
Sustained load factor for temperature range -40°C / +120°C			0,75							
Concrete cone failure										
Factor for uncracked concrete	$k_{ucr,N}$	[-]	11,0							
Edge distance	$c_{cr,N}$	[mm]	1,5 h_{ef}							
Spacing	$s_{cr,N}$	[mm]	3,0 h_{ef}							
Splitting failure										
Edge distance	$c_{cr,Nsp}$	[mm]	if $h = h_{min}$							
			$2,5 \cdot h_{ef}$	$2,0 \cdot h_{ef}$		$1,5 \cdot h_{ef}$				
			If $h_{min} < h < 2 \cdot h_{min}$							
			 interpolate values							
			if $h \geq 2 \cdot h_{min}$							
			$C_{cr,Np}$							
Spacing	$s_{cr,Nsp}$	[mm]	$2 \cdot C_{cr,sp}$							
Installation factor for combined pull-out, concrete cone and splitting failure										
Installation factor for category I1 [1)]	γ_{inst}	[-]	1,0							
Installation factor for category I2 [1)]			1,2							

[1)] In the absence of other national regulation

VI 100-PRO VI 100-PRO-W VI 100-PRO-T	**Annex C2** of European Technical Assessment ETA-14/0119
Performances Characteristic values for tension resistance in uncracked concrete under static and quasi-static loads – threaded rods	

Table C3: Characteristic values for tension resistance in cracked concrete under static and quasi-static loads – threaded rods. Working life of 50 and 100 years.

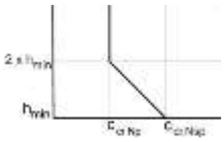
Size			M10	M12	M16	M20
Steel failure						
Characteristic resistance	$N_{Rk,s}$	[kN]	See Annex C1 – Table C1			
Partial factor	$\gamma_{Ms,N}$ [1)]	[-]	See Annex C1 – Table C1			
Combined pull-out and concrete cone failure in cracked concrete C20/25						
Characteristic bond resistance temperature range -40°C / +40°C	$\tau_{Rk,cr,50}$	[N/mm²]	9,0	9,0	9,0	6,5
Characteristic bond resistance temperature range -40°C / +80°C	$\tau_{Rk,cr,50}$	[N/mm²]	6,5	6,5	6,5	4,5
Characteristic bond resistance temperature range -40°C / +120°C	$\tau_{Rk,cr,50}$	[N/mm²]	3,5	3,5	3,5	2,5
Characteristic bond resistance temperature range -40°C / +40°C	$\tau_{Rk,cr,100}$	[N/mm²]	8,5	8,5	8,0	5,5
Characteristic bond resistance temperature range -40°C / +80°C	$\tau_{Rk,cr,100}$	[N/mm²]	6,0	6,0	5,5	4,0
Characteristic bond resistance temperature range -40°C / +120°C	$\tau_{Rk,cr,100}$	[N/mm²]	3,0	3,0	3,0	2,0
Increasing factor	ψ_c	[-]	$\left(\frac{f_{ck}}{20}\right)^{0.3}$			
Sustained load factor for temperature range -40°C / +40°C	ψ^0_{sus} $\psi^0_{sus,100}$	[-]	0,72			
Sustained load factor for temperature range -40°C / +80°C			0,74			
Sustained load factor for temperature range -40°C / +120°C			0,75			
Concrete cone failure						
Factor for cracked concrete	$k_{cr,N}$	[-]	7,7			
Edge distance	$c_{cr,N}$	[mm]	1,5 h_{ef}			
Spacing	$s_{cr,N}$	[mm]	3,0 h_{ef}			
Splitting failure						
Edge distance	$c_{cr,Nsp}$	[mm]	if $h = h_{min}$			
			$2{,}5 \cdot h_{ef}$	$2{,}0 \cdot h_{ef}$		$1{,}5 \cdot h_{ef}$
			If $h_{min} < h < 2 \cdot h_{min}$			
			 interpolate values			
			if $h \geq 2 \cdot h_{min}$			
			$C_{cr,Np}$			
Spacing	$s_{cr,Nsp}$	[mm]	$2 \cdot C_{cr,sp}$			
Installation factor for combined pull-out, concrete cone and splitting failure						
Installation factor for category I1 [1)]	γ_{inst}	[-]	1,0			
Installation factor for category I2 [1)]			1,2			

[1)] In the absence of other national regulation

Table C4: Characteristic values for shear resistance in uncracked and cracked concrete under static and quasi-static loads – threaded rods. Working life of 50 and 100 years.

<table>
<tr><th colspan="3">Size</th><th>M8</th><th>M10</th><th>M12</th><th>M16</th><th>M20</th><th>M24</th><th>M27</th><th>M30</th></tr>
<tr><td colspan="11">Steel failure without lever arm</td></tr>
<tr><td>Characteristic resistance</td><td>$V^0_{Rk,s}$</td><td>[kN]</td><td colspan="8">See Annex C1 – Table C1</td></tr>
<tr><td>Partial factor</td><td>$\gamma_{Ms,V}$ 1)</td><td>[-]</td><td colspan="8">See Annex C1 – Table C1</td></tr>
<tr><td>Ductility factor</td><td>k_7</td><td>[-]</td><td colspan="8">1,0</td></tr>
<tr><td colspan="11">Steel failure with lever arm</td></tr>
<tr><td>Characteristic resistance</td><td>$M^0_{Rk,s}$</td><td>[kN]</td><td colspan="8">See Annex C1 – Table C1</td></tr>
<tr><td>Partial factor</td><td>$\gamma_{Ms,V}$ 1)</td><td>[-]</td><td colspan="8">See Annex C1 – Table C1</td></tr>
<tr><td colspan="11">Concrete pry out failure</td></tr>
<tr><td>Factor</td><td>k_8</td><td>[-]</td><td colspan="8">2,0</td></tr>
<tr><td>Installation factor</td><td>γ_{inst}</td><td>[-]</td><td colspan="8">1,0</td></tr>
<tr><td colspan="11">Concrete edge failure</td></tr>
<tr><td>Effective length of anchor under shear loading</td><td>l_f</td><td>[-]</td><td colspan="6">$l_f = h_{ef}$ and $\leq 12\ d_{nom}$</td><td colspan="2">$l_f = h_{ef}$ and $\leq$ max (8 d_{nom}; 300 mm)</td></tr>
<tr><td>Installation factor</td><td>γ_{inst}</td><td>[-]</td><td colspan="8">1,0</td></tr>
<tr><td colspan="11">1) In the absence of other national regulation</td></tr>
</table>

Table C5: Characteristic values for tension resistance in uncracked concrete under static and quasi-static loads – rebar. Working life of 50 and 100 years.

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø28	Ø32
Steel failure											
Characteristic resistance	$N_{Rk,s}$	[kN]	A_s x f_{uk} [2]								
Cross section area	A_s	[mm²]	50	79	113	154	201	314	491	616	804
Partial factor	$\gamma_{Ms,N}$ [1]	[-]	1,4								
Combined pull-out and concrete cone failure in uncracked concrete C20/25											
Characteristic bond resistance temperature range -40°C / +40°C	$\tau_{Rk,ucr,50}$ $\tau_{Rk,ucr,100}$	[N/mm²]	14,0	13,0	13,0	12,0	10,0	9,5	9,5	8,5	7,5
Characteristic bond resistance temperature range -40°C / +80°C	$\tau_{Rk,ucr,50}$ $\tau_{Rk,ucr,100}$	[N/mm²]	10,0	9,5	9,0	9,0	7,5	7,0	7,0	6,0	5,5
Characteristic bond resistance temperature range -40°C / +120°C	$\tau_{Rk,ucr,50}$ $\tau_{Rk,ucr,100}$	[N/mm²]	5,5	5,0	5,0	5,0	4,0	4,0	4,0	3,5	3,0
Increasing factor	ψ_c	[-]	$(\frac{f_{ck}}{20})^{0.3}$								
Sustained load factor for temperature range -40°C / +40°C	ψ^0_{sus} $\psi^0_{sus,100}$	[-]	0,72								
Sustained load factor for temperature range -40°C / +80°C			0,74								
Sustained load factor for temperature range -40°C / +120°C			0,75								
Concrete cone failure											
Factor for uncracked concrete	$k_{ucr,N}$	[-]	11,0								
Edge distance	$c_{cr,N}$	[mm]	1,5 h_{ef}								
Spacing	$s_{cr,N}$	[mm]	3,0 h_{ef}								
Splitting failure											
Edge distance	$c_{cr,Nsp}$	[mm]	if $h = h_{min}$								
			2,5 · h_{ef}		2,0 · h_{ef}			1,5 · h_{ef}			
			If $h_{min} < h < 2 \cdot h_{min}$								
			 interpolate values								
			if $h \geq 2 \cdot h_{min}$								
			$C_{cr,Np}$								
Spacing	$s_{cr,Nsp}$	[mm]	2 · $C_{cr,sp}$								
Installation factor for combined pull-out, concrete cone and splitting failure											
Installation factor for category I1 [1]	γ_{inst}	[-]	1,0								
Installation factor for category I2 [1]			1,2								

[1] In the absence of other national regulation
[2] f_{uk} shall be taken from the specifications of reinforcing bars

Table C6: Characteristic values for shear resistance in uncracked concrete under static and quasi-static loads – rebar. Working life of 50 and 100 years.

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø28	Ø32
Steel failure without lever arm											
Characteristic resistance	$V^0_{Rk,s}$	[kN]	$0,5 \times A_s \times f_{uk}$ [2]								
Partial factor	$\gamma_{Ms,V}$ [1]	[-]	1,5								
Cross section area	A_s	[mm^2]	50	79	113	154	201	314	491	616	804
Ductility factor	k_7	[-]	1,0								
Steel failure with lever arm											
Characteristic resistance	$M^0_{Rk,s}$	[kN]	$1,2 \times W_{el} \times f_{uk}$ [2]								
Elastic section modulus	W_{el}	[mm^3]	50	98	170	269	402	785	1534	2155	3217
Partial factor	$\gamma_{Ms,V}$ [1]	[-]	1,5								
Concrete pry out failure											
Factor	k_8	[-]	2,0								
Installation factor	γ_{inst}	[-]	1,0								
Concrete edge failure											
Effective length of anchor under shear loading	l_f	[-]	$l_f = h_{ef}$ and $\leq 12\ d_{nom}$						$l_f = h_{ef}$ and $\leq \max (8\ d_{nom};$ 300 mm)		
Installation factor	γ_{inst}	[-]	1,0								

[1] In the absence of other national regulation
[2] f_{uk} shall be taken from the specifications of reinforcing bars

VI 100-PRO **VI 100-PRO-W** **VI 100-PRO-T**	**Annex C6** of European Technical Assessment ETA-14/0119
Performances Characteristic values for shear resistance in uncracked concrete under static and quasi-static loads – rebar	

Table C7. Displacement under tension loads for uncracked concrete under static and quasi-static loads – threaded rods.

Size			M8	M10	M12	M16	M20	M24	M27	M30
Characteristic displacement in uncracked concrete C20/25 to C50/60 under tension loads										
Service load [1)]	F	[kN]	9,6	10,8	14,3	23,8	29,6	42,4	40,4	44,4
Displacement	δ_{N0}	[mm]	0,30	0,30	0,35	0,35	0,35	0,40	0,40	0,45
	$\delta_{N\infty}$	[mm]	0,85	0,85	0,85	0,85	0,85	0,85	0,85	0,85

Table C8: Displacement under tension loads for cracked concrete under static and quasi-static loads – threaded rods.

Size			M10	M12	M16	M20
Characteristic displacement in cracked concrete C20/25 to C50/60 under tension loads						
Service load [1)]	F	[kN]	9,5	14,3	21,4	23,8
Displacement	δ_{N0}	[mm]	0,50	0,50	0,70	0,60
	$\delta_{N\infty}$	[mm]	0,85	0,85	0,85	0,85

Table C9: Displacement under shear loads for uncracked and cracked concrete under static and quasi-static loads – threaded rods.

Size			M8	M10	M12	M16	M20	M24	M27	M30
Characteristic displacement in cracked and uncracked concrete C20/25 to C50/60 under shear loads										
Service load [1)]	F	[kN]	3,7	5,8	8,4	15,7	24,5	35,3	45,5	55,6
Displacement	δ_{V0}	[mm]	2,0	2,0	2,0	2,0	2,0	2,0	2,0	2,0
	$\delta_{V\infty}$	[mm]	3,0	3,0	3,0	3,0	3,0	3,0	3,0	3,0

Table C10: Displacement under tension loads for uncracked concrete under static and quasi-static loads – rebar.

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø28	Ø32
Characteristic displacement in uncracked concrete C20/25 to C50/60 under tension loads											
Service load [1)]	F	[kN]	10,1	13,6	17,2	20,1	23,9	41,2	53,3	64,1	67,3
Displacement	δ_{N0}	[mm]	0,33	0,33	0,40	0,41	0,42	0,45	0,45	0,47	0,48
	$\delta_{N\infty}$	[mm]	0,85	0,85	0,85	0,85	0,85	0,85	0,85	0,85	0,85

Table C11: Displacement under shear loads for uncracked concrete under static and quasi-static loads – rebar.

Size			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	Ø28	Ø32
Characteristic displacement in uncracked concrete C20/25 to C50/60 under shear loads											
Service load [1)]	F	[kN]	13,2	20,6	29,6	40,3	52,7	82,3	128,6	161,3	210,6
Displacement	δ_{V0}	[mm]	2,0	2,0	2,0	2,0	2,0	2,0	2,0	2,0	2,0
	$\delta_{V\infty}$	[mm]	3,0	3,0	3,0	3,0	3,0	3,0	3,0	3,0	3,0

[1)] These values are suitable for each temperature range and categories specified in Annex B1

Table C12: Characteristic values for tension resistance for seismic performance category C1 – threaded rods. Working life of 50 and 100 years.

Size			M12	M16	M20
Steel failure					
Characteristic resistance	$N_{Rk,s,eq,C1}$	[kN]	1,0 x $N_{Rk,s}$		
Partial factor [1]	$\gamma_{Ms,N}$	[-]	See Annex C1 – Table C1		
Combined pull-out and concrete cone failure					
Characteristic bond resistance temperature range -40°C / +40°C	$\tau_{Rk,C1}$	[N/mm²]	4,2	3,7	3,7
Characteristic bond resistance temperature range -40°C / +80°C	$\tau_{Rk,C1}$	[N/mm²]	3,0	2,7	2,7
Characteristic bond resistance temperature range -40°C / +120°C	$\tau_{Rk,C1}$	[N/mm²]	1,6	1,4	1,4
Increasing factor for C30/37	ψ_c	[-]	1,0		
Increasing factor for C40/50					
Increasing factor for C50/60					
Installation factor for category I1 [1]	γ_{inst}	[-]	1,0		
Installation factor for category I2 [1]			1,2		

[1] In the absence of other national regulation

Table C13: Characteristic values for shear resistance for seismic performance category C1 – threaded rods. Working life of 50 and 100 years.

Size			M12	M16	M20
Steel failure					
Characteristic resistance	$V_{Rk,s,eq,C1}$	[kN]	0,7 x $V^0_{Rk,s}$		
Partial factor [1]	$\gamma_{Ms,V}$	[-]	See Annex C1 – Table C1		

[1] In the absence of other national regulation

Table C14: Reduction factor for annular gap. Working life of 50 and 100 years.

Reduction factor for annular gap			
Without annular gap filling	α_{gap}	[-]	0,5
With annular gap filling	α_{gap}	[-]	1,0

Table C15: Characteristic values for tension resistance for seismic performance category C2 – threaded rods. Working life of 50 and 100 years.

Size			M12	M16
Steel failure				
Characteristic resistance	$N_{Rk,s,eq,C2}$	[kN]	1,0 x $N_{Rk,s}$	
Partial factor [1)]	$\gamma_{Ms,N}$ [1)]	[-]	See Annex C1 – Table C1	
Combined pull-out and concrete cone failure				
Characteristic bond resistance temperature range -40°C / +40°C	$\tau_{Rk,eq,C2}$	[N/mm²]	1,6	1,7
Characteristic bond resistance temperature range -40°C / +80°C	$\tau_{Rk,eq,C2}$	[N/mm²]	1,2	1,2
Characteristic bond resistance temperature range -40°C / +120°C	$\tau_{Rk,eq,C2}$	[N/mm²]	0,6	0,7
Increasing factor for C30/37	ψ_c	[-]	1,0	
Increasing factor for C40/50				
Increasing factor for C50/60				
Installation factor for category I1 [1)]	γ_{inst}	[-]	1,0	
Installation factor for category I2 [1)]			1,2	
[1)] In the absence of other national regulation				

Table C16: Characteristic values for shear resistance for seismic performance category C2 – threaded rods. Working life of 50 and 100 years.

Size			M12	M16
Steel failure				
Characteristic shear resistance	$V_{Rk,s,eq,C2}$	[kN]	0,53 x $V^0_{Rk,s}$	0,46 x $V^0_{Rk,s}$
Partial factor [1)]	$\gamma_{Ms,V}$	[-]	See Annex C1 – Table C1	
[1)] In the absence of other national regulation				

Table C17: Reduction factor for annular gap. Working life of 50 and 100 years.

Reduction factor for annular gap			
Without annular gap filling	α_{gap}	[-]	0,5
With annular gap filling	α_{gap}	[-]	1,0

Table C18: Displacements under tensile and shear loads for seismic performance category C2 – threaded rods.

Size			M12	M16
Displacements for tensile and shear load for seismic performance category C2				
Displacement in tensile at damage limitation state	$\delta_{N,eq,C2\,(DLS)}$	[mm]	0,20	0,23
Displacement in tensile at ultimate limit state	$\delta_{N,eq,C2\,(ULS)}$	[mm]	0,33	1,04
Displacement in shear at damage limitation state	$\delta_{V,eq,C2\,(DLS)}$	[mm]	2,01	0,70
Displacement in shear at ultimate limit state	$\delta_{V,eq,C2\,(ULS)}$	[mm]	4,68	2,12

Characteristic bond resistance of a single bonded fastener $\tau_{Rk,fi,p}(\theta)$ for concrete strength classes C20/25 to C50/60 with all drilling methods under fire conditions for working life of 50 and 100 years.

The characteristic bond resistance of a single bonded fastener under fire conditions $\tau_{Rk,fi,p}$ for a given temperature (θ) shall be calculated using the following equations:

$$\tau_{Rk,fi,p}(\theta) = k_{fi,p}(\theta) * \tau_{Rk,cr,C20/25}$$

$$\tau_{Rk,fi,p}(\theta) = k_{fi,p}(\theta) * \tau_{Rk,cr,100,C20/25}$$

Where:

$$if\ \theta \leq \theta_{max} \quad k_{fi,p}(\theta) = k_{fi,p}(\theta) = 0{,}8049 \cdot e^{-0{,}0097 \cdot \theta} \leq 1{,}0$$

$$if\ \theta > \theta_{max} \quad k_{fi,p}(\theta) = k_{fi,p}(\theta) = 0$$

θ_{max} = 271°C

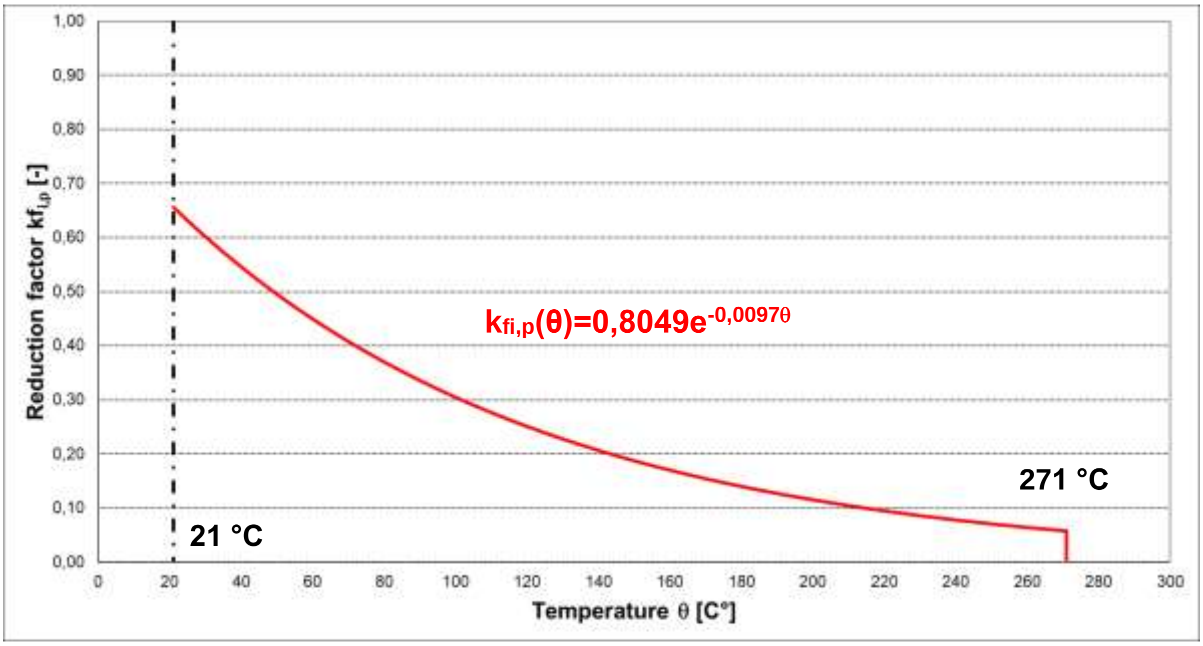
$\tau_{Rk,fi,p}$ = characteristic bond resistance for cracked concrete under fire exposure for a given temperature (θ)

$k_{fi,p(\theta)}$ = reduction factor for bond resistance under fire exposure

$\tau_{Rk,cr,C20/25}$ = characteristic bond resistance for cracked concrete for concrete strength class C20/25 for a working life of 50 years given in Table C3.

$\tau_{Rk,cr,100,C20/25}$ = characteristic bond resistance for cracked concrete for concrete strength class C20/25 for a working life of 100 years given in Table C3.

Figure C1: Reduction factor $k_{fi,p}(\theta)$



VI 100-PRO VI 100-PRO-W VI 100-PRO-T	**Annex C10** of European Technical Assessment ETA-14/0119
Performances Reduction factor for pull-out failure of single fasteners under fire conditions – threaded rods	

Table C19: Characteristic resistance under tension load in case of steel failure under fire conditions – threaded rods.

Size			M10	M12	M16	M20
Steel failure						
Steel class 5.8 to 8.8	$N_{Rk,s,fi\ (30)}$	[kN]	0,87	1,70	3,14	4,90
	$N_{Rk,s,fi\ (60)}$	[kN]	0,75	1,28	2,36	3,68
	$N_{Rk,s,fi\ (90)}$	[kN]	0,58	1,11	2,04	3,19
	$N_{Rk,s,fi\ (120)}$	[kN]	0,46	0,85	1,57	2,45
Stainless steel A4	$N_{Rk,s,fi\ (30)}$	[kN]	1,45	2,55	4,71	7,35
	$N_{Rk,s,fi\ (60)}$	[kN]	1,16	2,13	3,93	6,13
	$N_{Rk,s,fi\ (90)}$	[kN]	0,93	1,70	3,14	4,90
	$N_{Rk,s,fi\ (120)}$	[kN]	0,81	1,36	2,51	3,92

Table C20: Characteristic resistance under shear load with and without lever arm in case of steel failure under fire conditions – threaded rods.

Size			M10	M12	M16	M20
Steel failure						
Steel class 5.8 to 8.8	$V_{Rk,s,fi\ (30)}$	[kN]	0,87	1,70	3,14	4,90
	$V_{Rk,s,fi\ (60)}$	[kN]	0,75	1,28	2,36	3,68
	$V_{Rk,s,fi\ (90)}$	[kN]	0,58	1,11	2,04	3,19
	$V_{Rk,s,fi\ (120)}$	[kN]	0,46	0,85	1,57	2,45
Stainless steel A4	$V_{Rk,s,fi\ (30)}$	[kN]	1,45	2,55	4,71	7,35
	$V_{Rk,s,fi\ (60)}$	[kN]	1,16	2,13	3,93	6,13
	$V_{Rk,s,fi\ (90)}$	[kN]	0,93	1,70	3,14	4,90
	$V_{Rk,s,fi\ (120)}$	[kN]	0,81	1,36	2,51	3,92
Steel class 5.8 to 8.8	$M_{Rk,s,fi\ (30)}$	[Nm]	1,1	2,7	6,7	13,0
	$M_{Rk,s,fi\ (60)}$	[Nm]	1,0	2,0	5,0	9,7
	$M_{Rk,s,fi\ (90)}$	[Nm]	0,7	1,7	4,3	8,4
	$M_{Rk,s,fi\ (120)}$	[Nm]	0,6	1,3	3,3	6,5
Stainless steel A4	$M_{Rk,s,fi\ (30)}$	[Nm]	1,9	4,0	10,0	19,5
	$M_{Rk,s,fi\ (60)}$	[Nm]	1,5	3,3	8,3	16,2
	$M_{Rk,s,fi\ (90)}$	[Nm]	1,2	2,7	6,7	13,0
	$M_{Rk,s,fi\ (120)}$	[Nm]	1,0	2,1	5,3	10,4

Table C21: Characteristic resistance under tension load in case of concrete cone and splitting failure under fire conditions – threaded rods.

Size			M10	M12	M16	M20
Concrete cone failure						
Steel class 5.8 to 8.8 Stainless steel A4	$N^0_{Rk,c,fi\,(30)}$	[kN]	$\frac{h_{ef}}{200} \cdot N^0_{Rk,c} \leq N^0_{Rk,c}$			
	$N^0_{Rk,c,fi\,(60)}$	[kN]				
	$N^0_{Rk,c,fi\,(90)}$	[kN]				
	$N^0_{Rk,c,fi\,(120)}$	[kN]	$0{,}8 \cdot \frac{h_{ef}}{200} \cdot N^0_{Rk,c} \leq N^0_{Rk,c}$			
Characteristic spacing	$s_{cr,N,fi}$	[mm]	$4 \cdot h_{ef}$			
Characteristic edge distance	$c_{cr,N,fi}$	[mm]	$2 \cdot h_{ef}$			

Table C22: Characteristic resistance under shear load in case of pry-out failure under fire conditions – threaded rods.

Size			M10	M12	M16	M20
Pryout failure						
Steel class 5.8 to 8.8 Stainless steel A4	$V_{Rk,cp,fi\,(30)}$	[kN]	$k_8 \cdot N_{Rk,c,fi\,(90)}$			
	$V_{Rk,cp,fi\,(60)}$	[kN]				
	$V_{Rk,cp,fi\,(90)}$	[kN]				
	$V_{Rk,cp,fi\,(120)}$	[kN]	$k_8 \cdot N_{Rk,c,fi\,(120)}$			

Table C23: Characteristic resistance under shear load in case of concrete edge failure under fire conditions – threaded rods.

Size			M10	M12	M16	M20
Concrete edge failure						
Steel class 5.8 to 8.8 Stainless steel A4	$V_{Rk,c,fi\,(30)}$	[Nm]	$0{,}25 \cdot V^0_{Rk,c}$			
	$V_{Rk,c,fi\,(60)}$	[Nm]				
	$V_{Rk,c,fi\,(90)}$	[Nm]				
	$V_{Rk,c,fi\,(120)}$	[Nm]	$0{,}20 \cdot V^0_{Rk,c}$			